material survey





ull masonry

of San Vigilio

support









iron carpentry elements



THE MASONRY INVESTIGATION



masonry analysis



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stabilized mixed coating with brick rubble 0.0-1.0-	
grey sandy silt 1.2-1.4– grey sandy silt 1.2-1.4– gravel with light brown sand and rubble 1.4-2.0– silty clay 2.0-2.3–	
SECTION fine silt sand with gravel and brick rubble 2.3-3.0—	
medium sand with fine and medium gravel 3.0-7.7—	
medium sand with fine gravel 7.7-9.0-	
medium sand with fine gravel 9.0-9.9-	
fine and medium sand 10.2-15.3-	
medium light brown sand 16.6-20.1—	
medium grey sand 20.1-22.5-	
grey sandy silt 22.5-23.0— fine sand weakly silty aray 23.0-23.4—	
silt weakly sandy grey 23.4-24.4-	
grey silt 24.4-25.7-	
grey sandy silt 25.7-27.3-	
fine grey silt sand 27.3-30.0-	
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MATRIX:	Uneven, characterized by a strong heterogeneity and widespread macro and micro porosity.
COLOUR:	Varied, due to the inhomogeneity of dough and cooking. Tendentially reddish and blackish.
dimension:	Various, mainly about 5.5x12 x24 cm. Some bricks have a longer length, reaching 27/28 cm
TEXTURE:	Arrangement to regular and staggered horizontal courses of bricks predominantly of band, alternating with header ones.
	LIME MORTAR
MATRIX:	Homogeneous, consisting of lime and aggregate of various grain sizes and colors typical of the sands coming from the lake. The mixtures used have a high inert binding ratio and in some places there are traces of cocciopesto.
COLOUR:	Generally light gray
dimension:	Height horizontal joints 2.5cm, width vertical joints
TEXTURE:	Distance vertical joints 8cm, distance horizontal joints uneven

NB: The types of stilatura realized originally and during the preservation interventions over the years don't give the possility to clearly define the profile of the bricks inside the masonry, that have been retraced in this analysis following the drawing of the mortar.

laboratory analysis on the critical section

direct shear test Atterberg limits consolidation coefficient hedometric consolidation test

A series of laboratory analysis has been developed on the test taken from the critical section found during the geognostic verifications. Considering the absence of evident out of plumb and an apparent homogeneus distribution of the loads, it can be assumed that this level has an uniform developement. Considering the small dimension of the test withdrawned and its dishomogeneity it has been developed ad unconsolidated, undrained shear test instead of a triaxial test. The others tests have been developed regularly.

The results of the tests, analized together, show a good degree of over-consoling and a permeability typical for this type of soil. In general, the results of the edometric tests have shown a fair degree of over-consolidation due to the load imparted by the structure and fair characteristics of bearing capacity, compatible with the loads to which the soil is subjected.



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90's investigations

geological investigations

CRITICAL SECTION | 9.9-10.2m weakly silty clay with sand levels

The investigations were carried out on an S1 test extracted in the depth range 9.9-10.2m, in the only layer significant from the point of view of the danger identified during the geological investigations: about 30-40cm of silty clays alternating with centimetric layers of sand.





laboratory analysis on bricks bricks microscope inspection Const A 6 10 15 10 13

porosity measurements

13

5 samples were taken from as many points in the tower. The specimens were dried at 105 ° and weighed, before being subjected to immersion, boiling and reweighing.

The microscope inspection of the bricks tests collected all around the masonry and the porosity measurments have showed how effective porosity of each is likely different on the basis of different compositions, which resulted in a diversified response to atmospheric agents. In general, the values relating to the surface portions, most exposed to degradation sources, showed greater porosity and lower density.

load tests with flat jacks

12 tests were performed with the aim to identify the maximum pressure beyond which lesions form in the portions of masonry adjacent to the shear. The data obtained have allowed to reach informations about the load-bearing capacity of the masonry and on its effective originality.

NB: The values shown are the result of tests carried out in the 1990s, with techniques that are now unusual compared to current potential. For this reason they must be considered the basis for further investigations necessary to obtain a more realistic point of view of the state of the masonry.

deepening hypotesis

On the basis of the data collected during the surveys carried out in the '90s and during the personal direct survey of the Tower, it is possible to hypothesize a new survey project that will allow to analyze exhaustively all the questions that have remained unresolved as a result of the investigations already carried out and the problems that have emerged in recent years.

The following is therefore a hypothesis of deepening with respect to what, over the years, by different people and with different techniques, can be considered as already evaluated and still valid, although in some cases, as has been better explained, the data collected should be used critically as the techniques used to obtain them are now obsolete and sometimes even questionable.

single flat jack tests

double flat jack tests





discontinuities and modified ares inspection



cracking static and dynamic monitoring



top area inspection



