

City Approach: Milan



The Wall of Porta Romana The BEIC Library in Milan

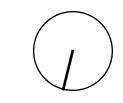


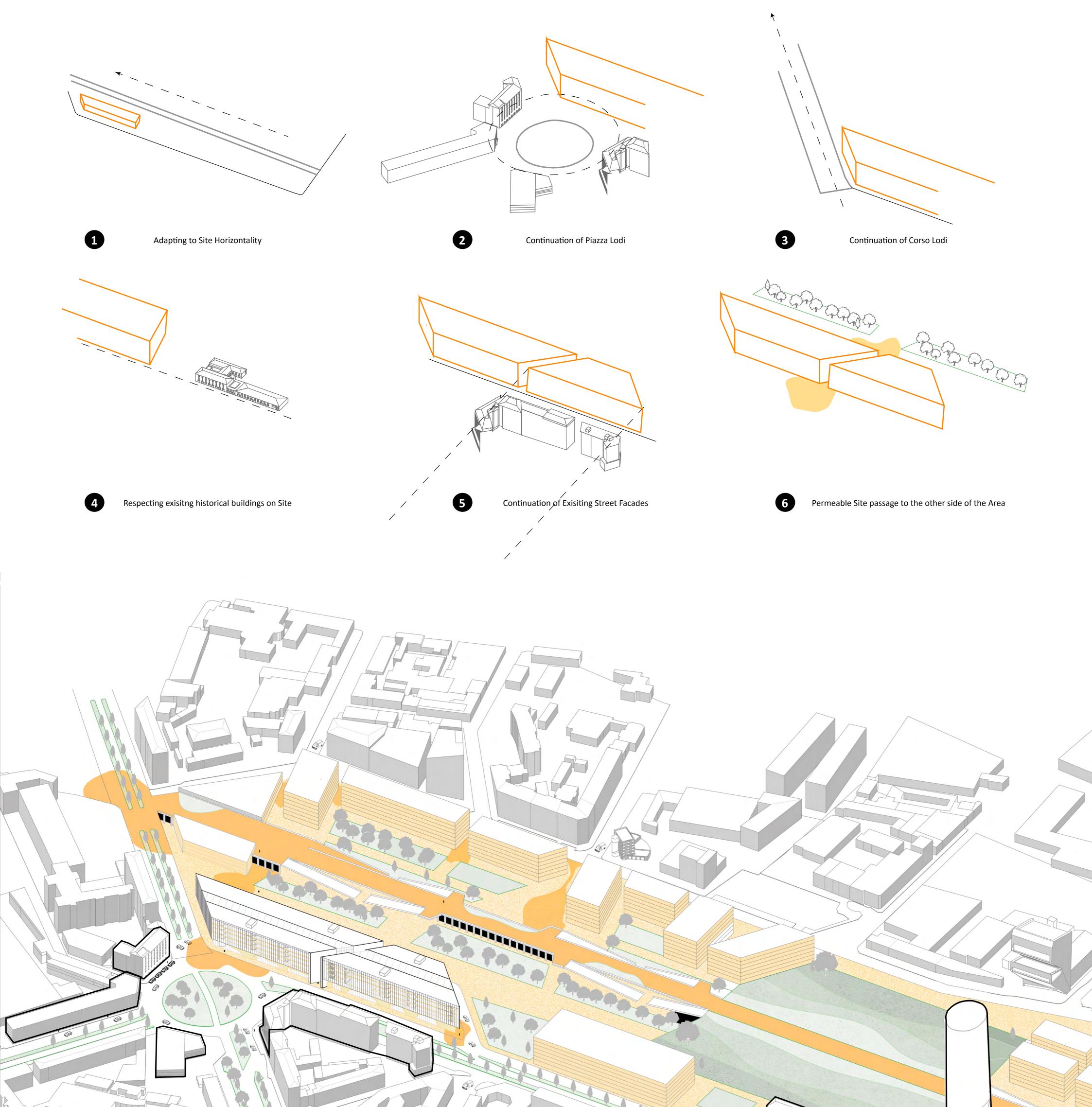
Location in the City of Milan

Maria Grazia Folli Corrado Pecora Giovanni Dotelli Marco Imperadori Lorenzo Pagliano Politecnico Di Milano_ AUIC School Architecture: Building Architecture_Thesis Architectural Studio of Complex Construction II Academic Year 2022/2023



Urban Strategy: Area Continuation





Regeneration of Porta Romana Site through a fully equipted Master Plan. *Focusing on Permeability as a goal.*

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Urban Strategy

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Se.

Maria Grazia Folli

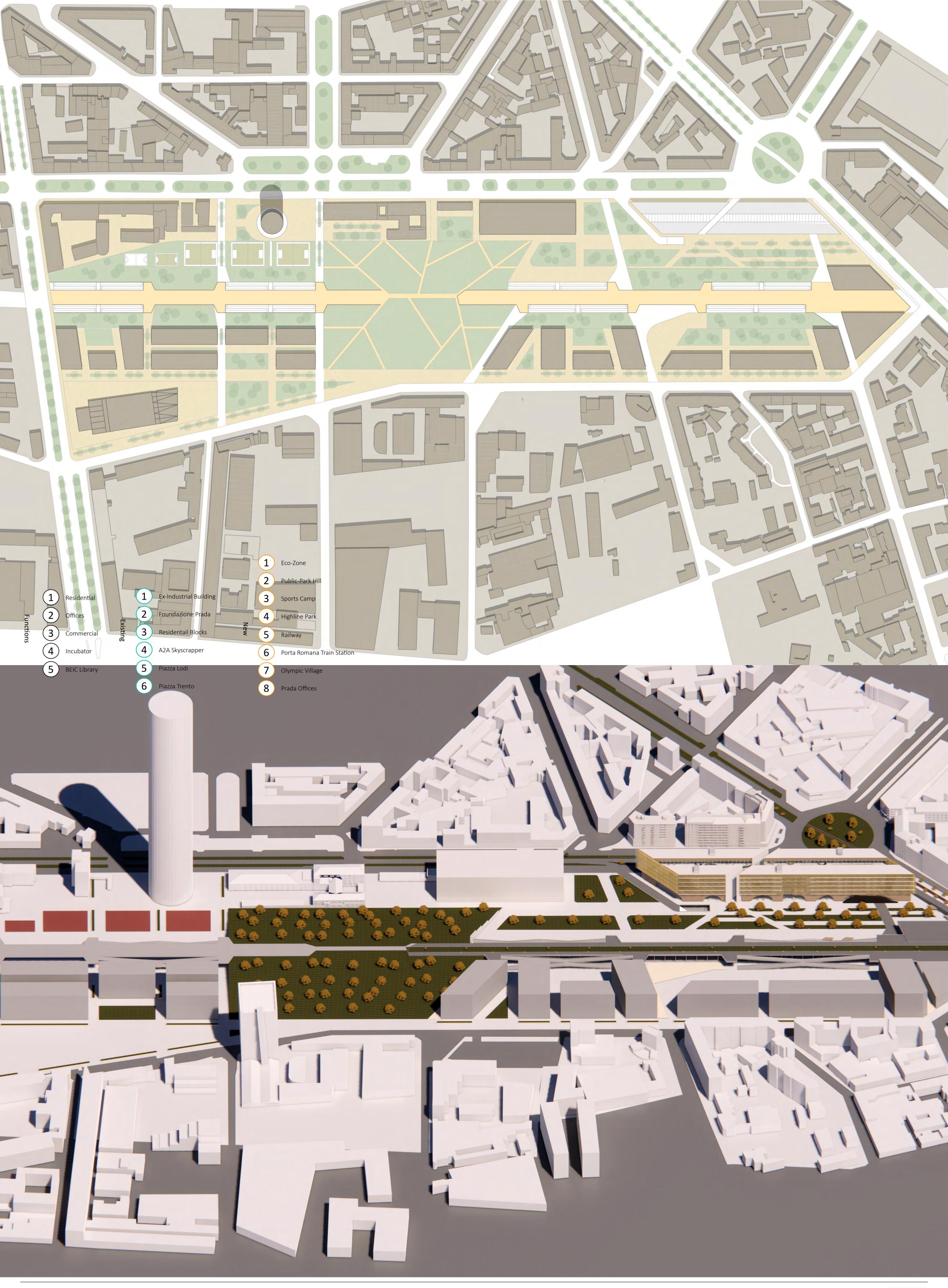
Corrado Pecora

Giovanni Dotelli

Marco Imperadori

Lorenzo Pagliano





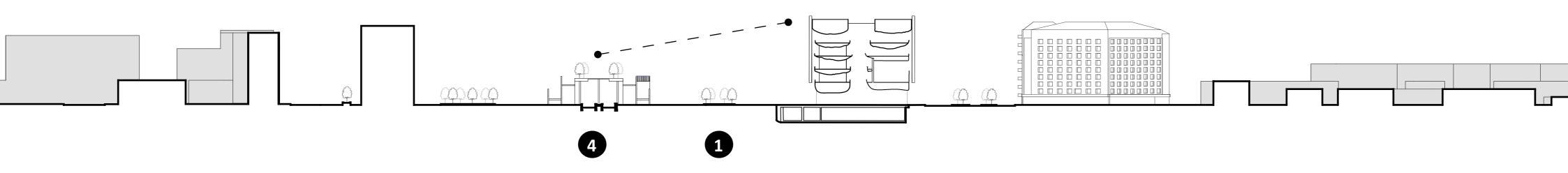
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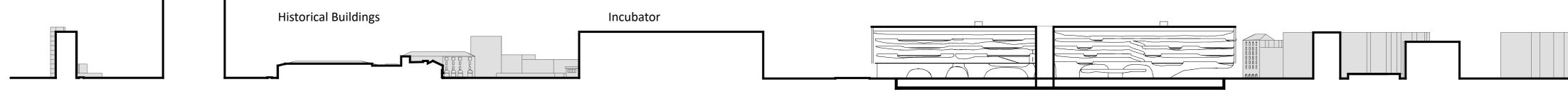




2 Bike Stations







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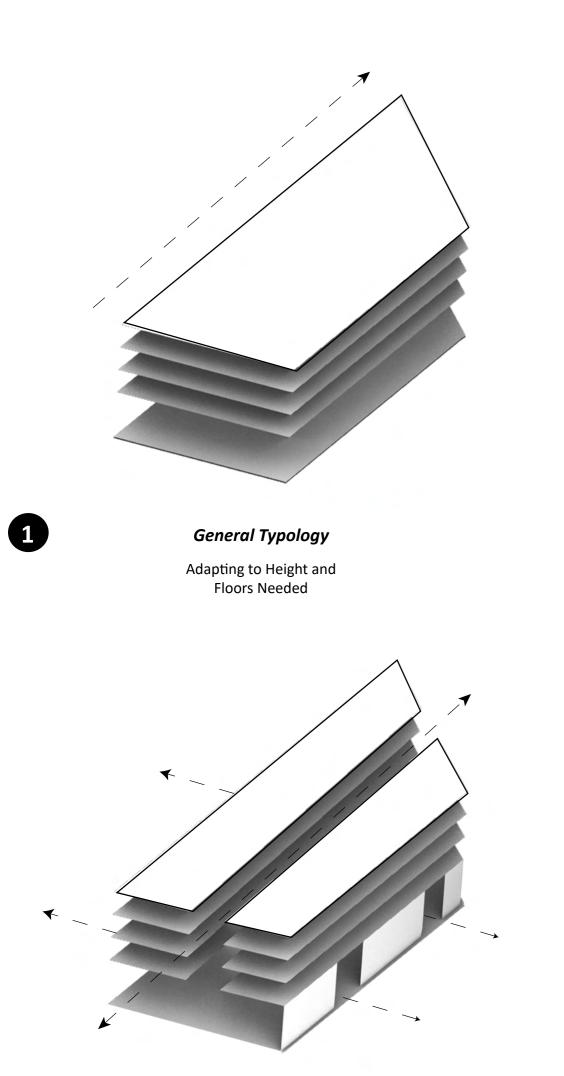


Masterplan Design

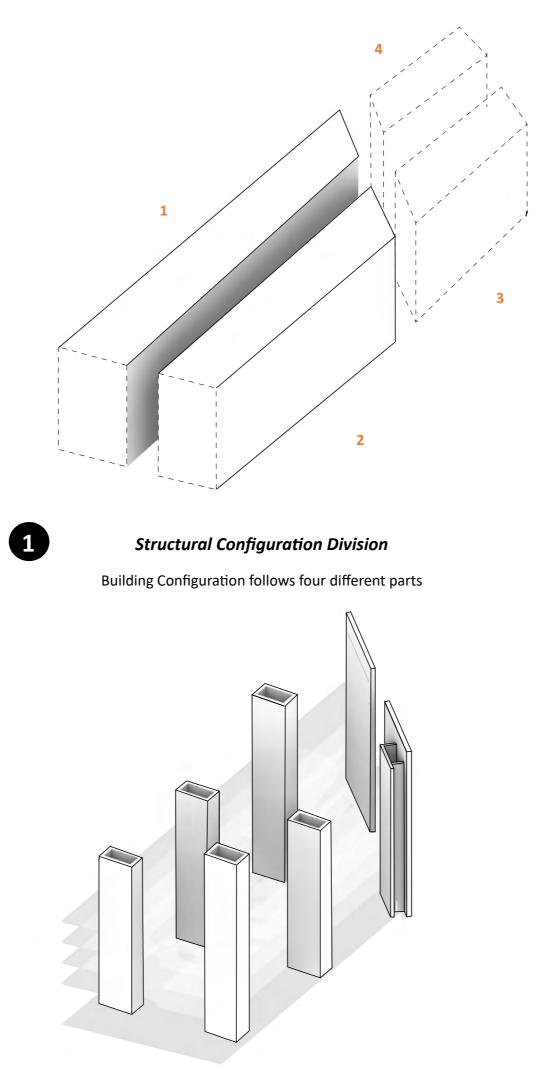
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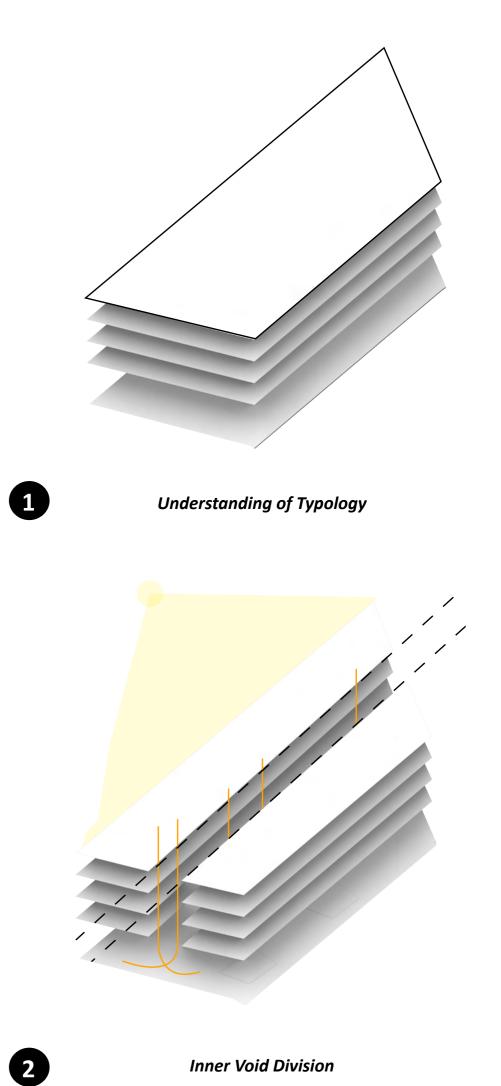
Concept and Composition: Architecture



Concept and Composition: Structure



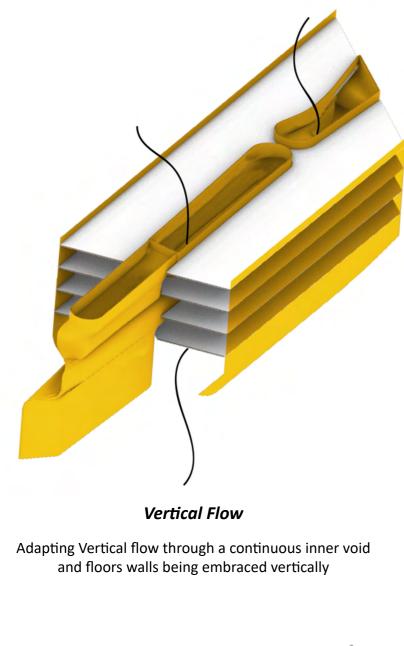
Concept and Composition: Technology





3

Horizontal Flow Adapting Horizontal Flow in the Middle Void and an open ground floor







Core and Shear Wall Distribution

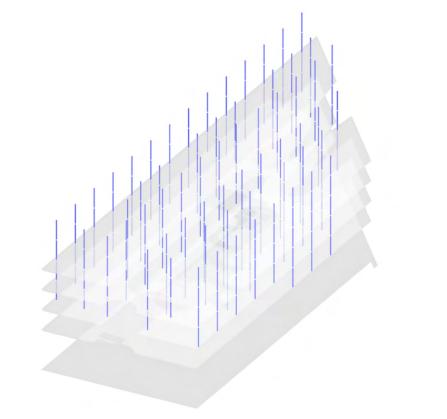
Shear and Cores heavily distributed around the edges of the building



4

Cores Carried By Rooftop Truss

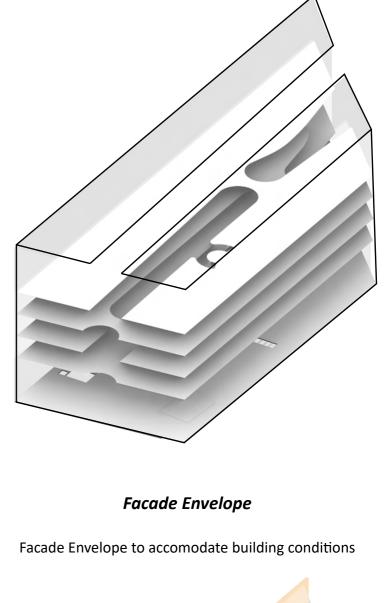
Rooftop truss and special trusses were diributed as the main structural carrier.

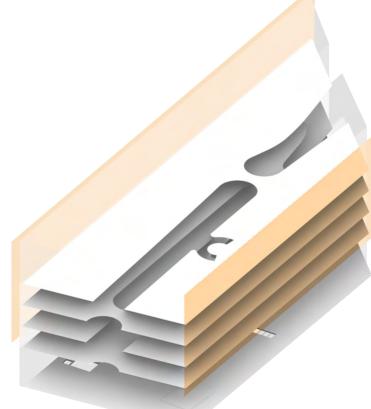


Cables Suspended From Truss

Inner Void Division

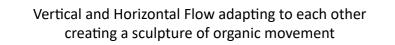
Adapting to Ventilation and Light conditions



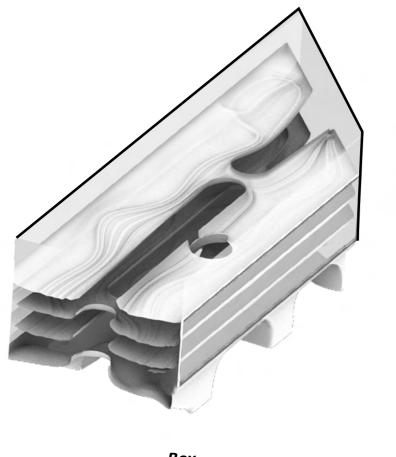


Design of Facade System

Adapting to inner functions and exterior conditions

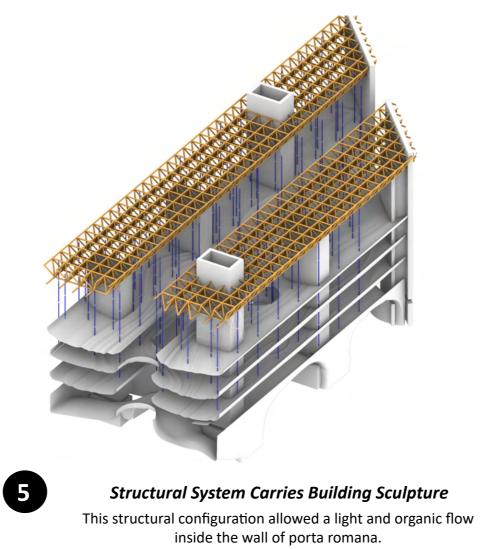


Sculpture



Вох Facade taking a role of a calm element of a box which is adapting to it's presence in the surrounding

Narrow cables suspended from the trusses





3

4

The possibility of creation of the flow Flow is created in a form of organic form through developed procedures of construction.

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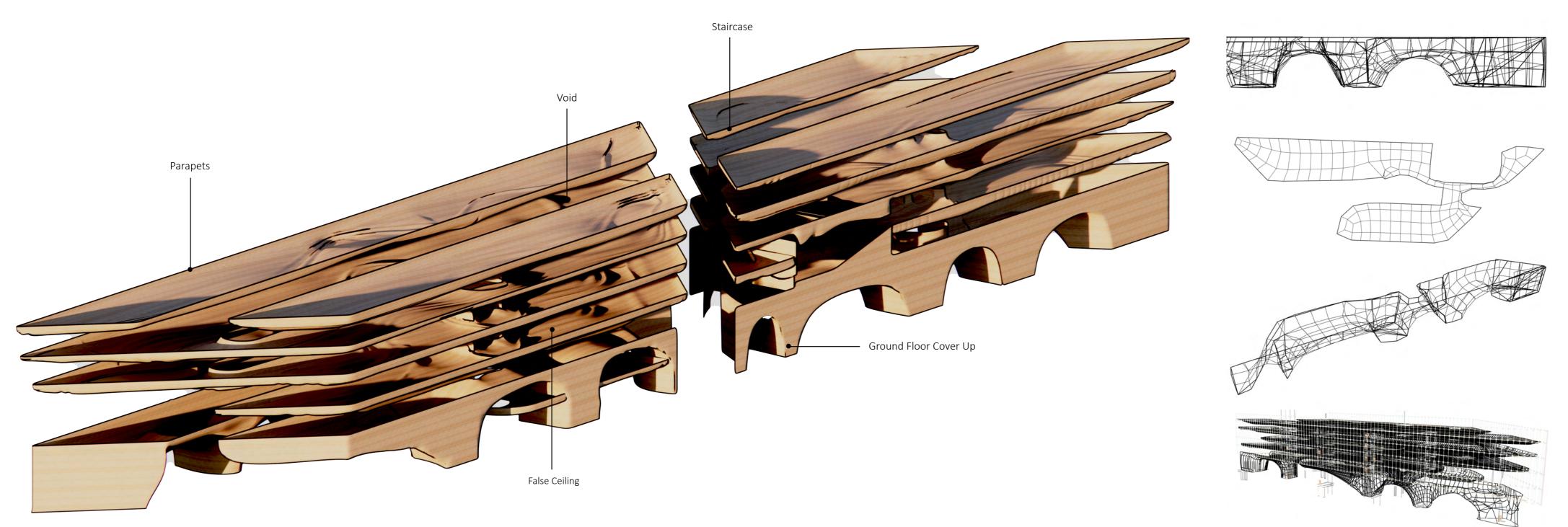
06

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The Sculpture: Composition

Process of Thinking





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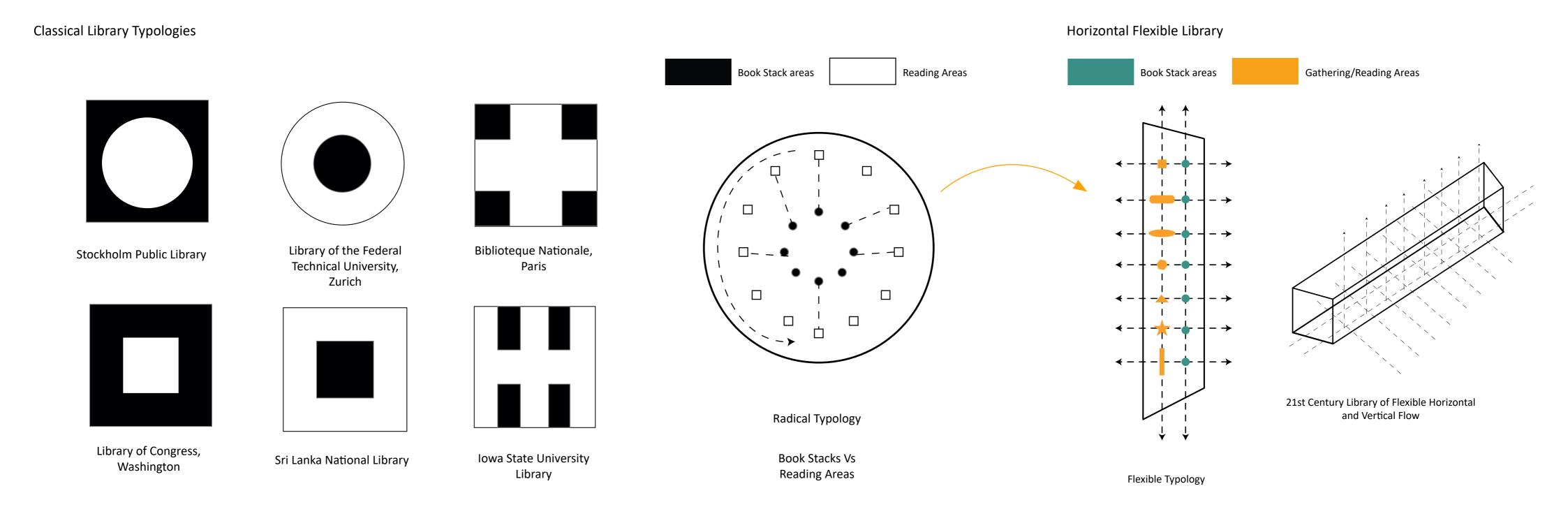


Interior: Sculpture

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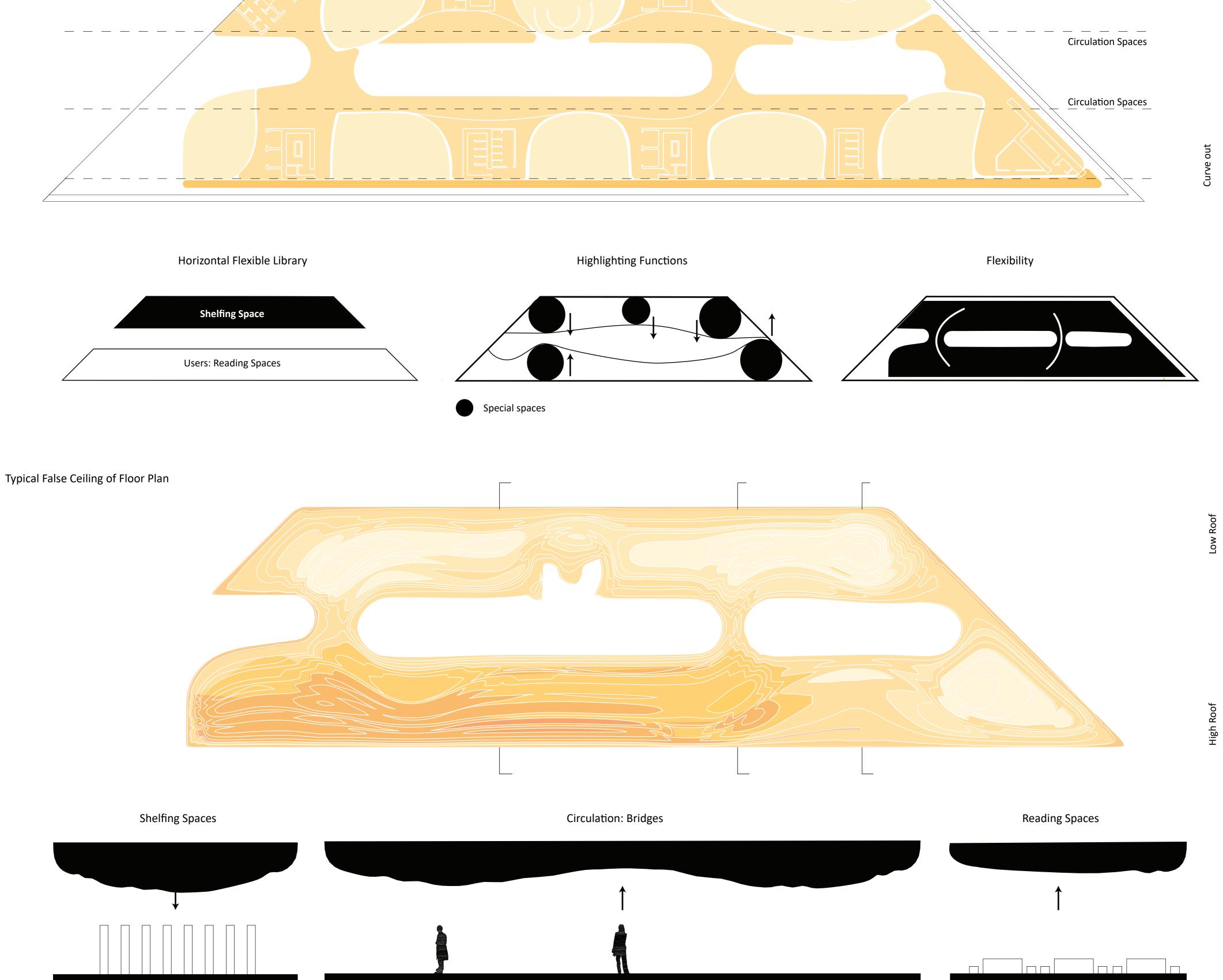


Building Typology Concept: From Typical Library Typology to Fluid Horizontal Typology



Re-Interpretation of Flow in Library Plan Typology and False Ceiling Typology

Typical Floor Plan



Curve out

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09

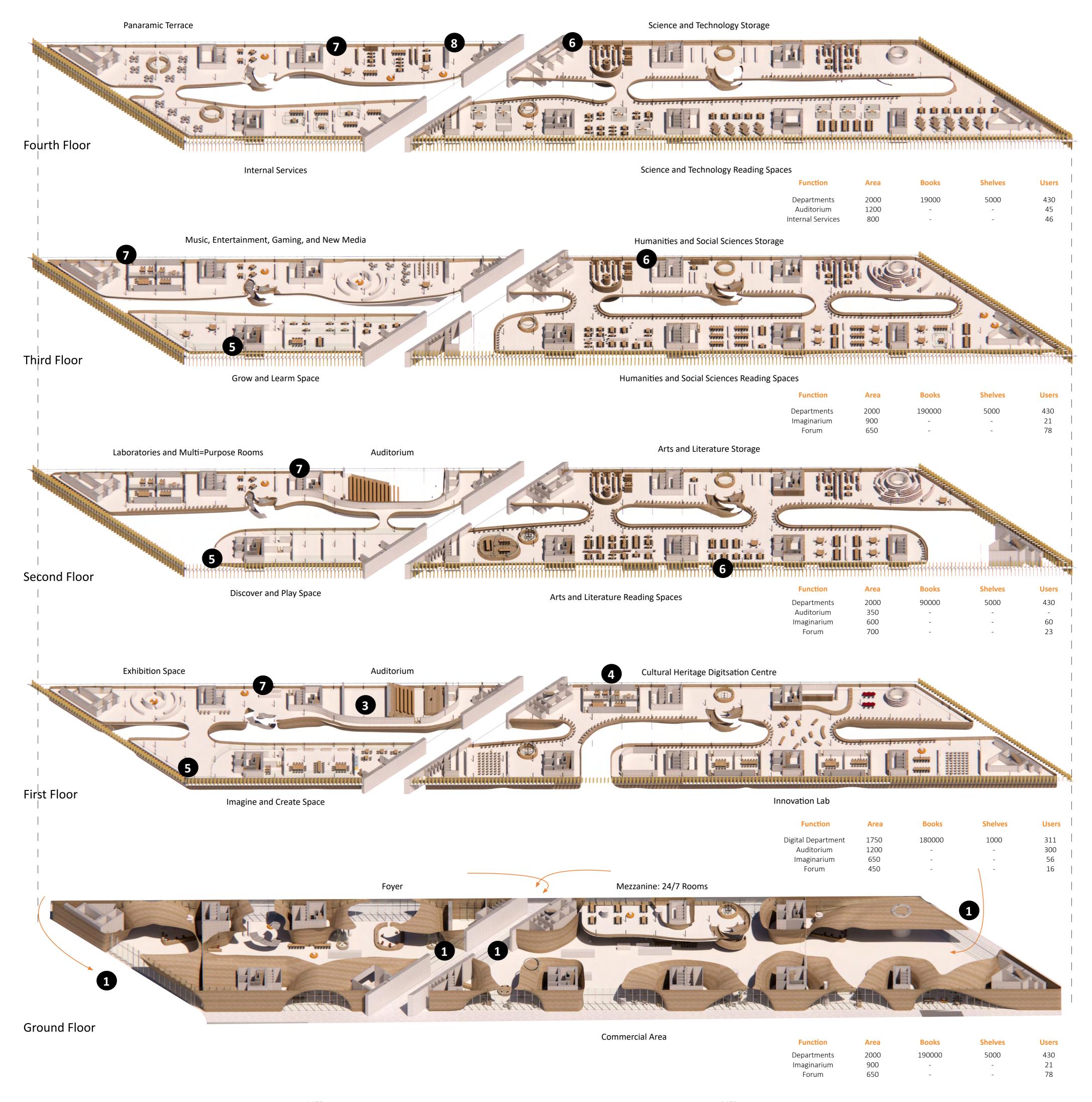
Plan and False Ceiling Typology

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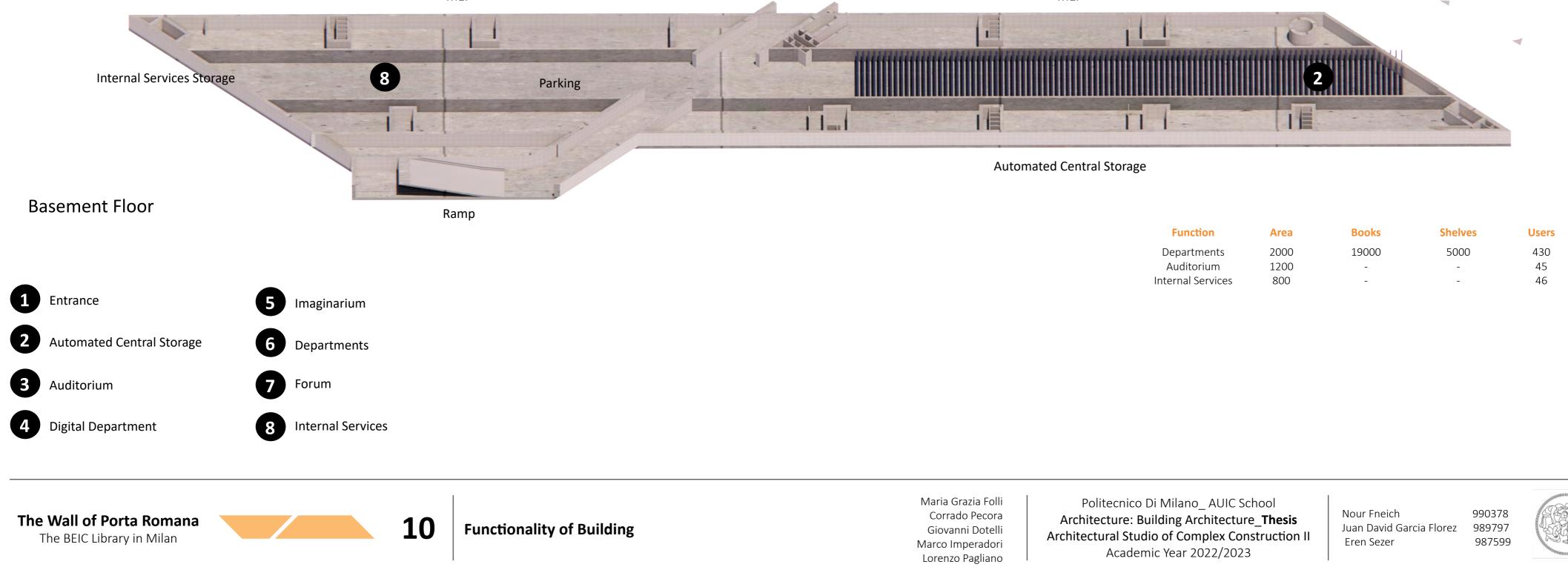


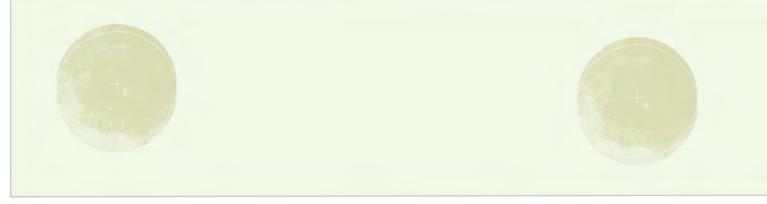
Functional Distribution Throughout Building

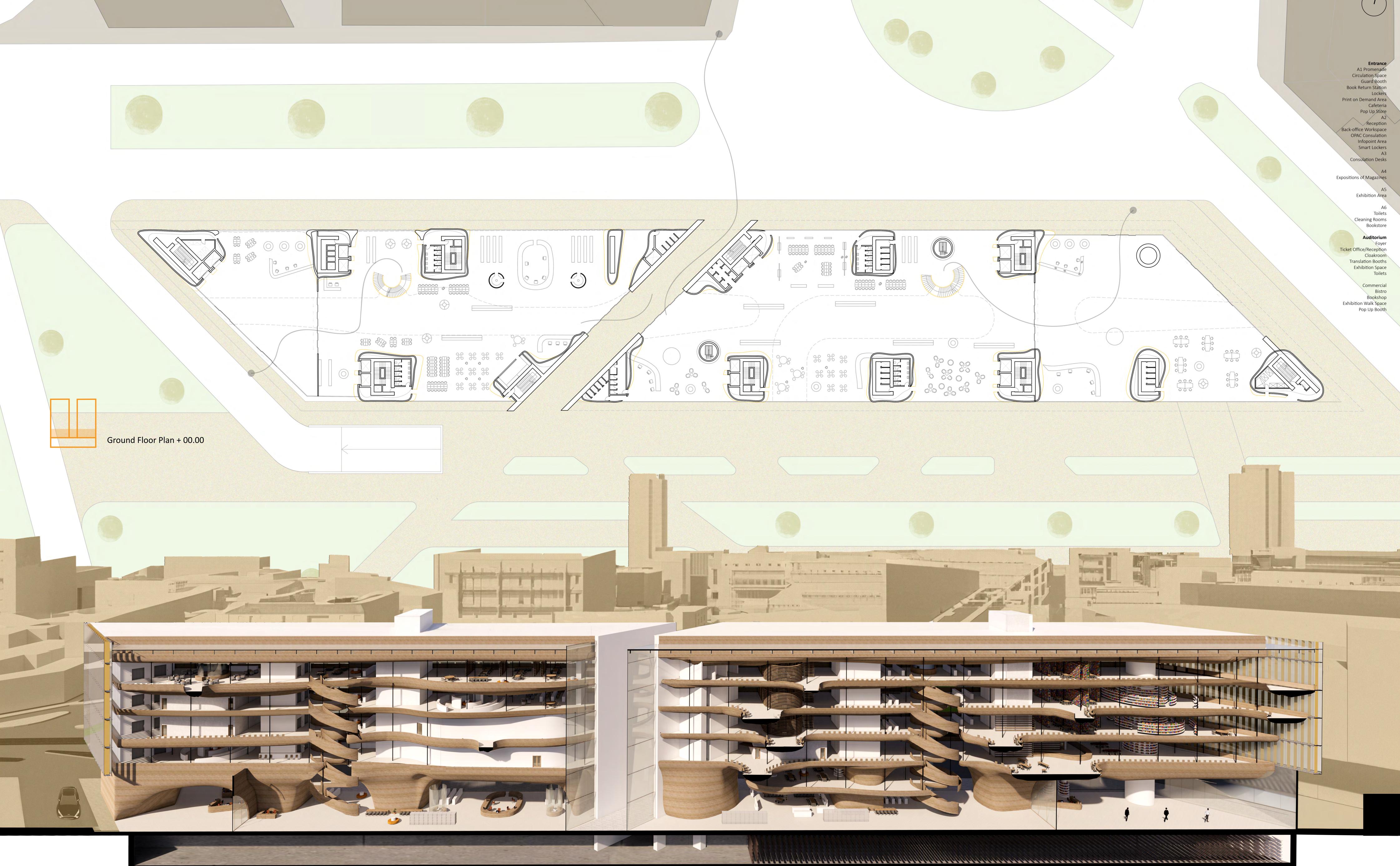


MEP

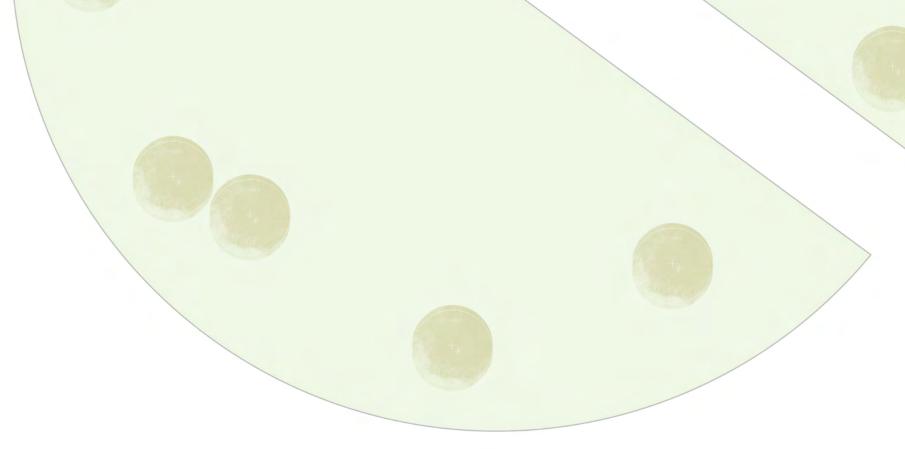
MEP

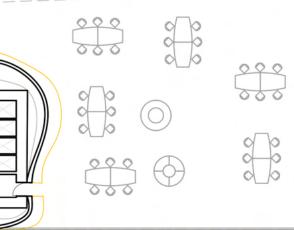






Perspective Transversal Section through the Atrium



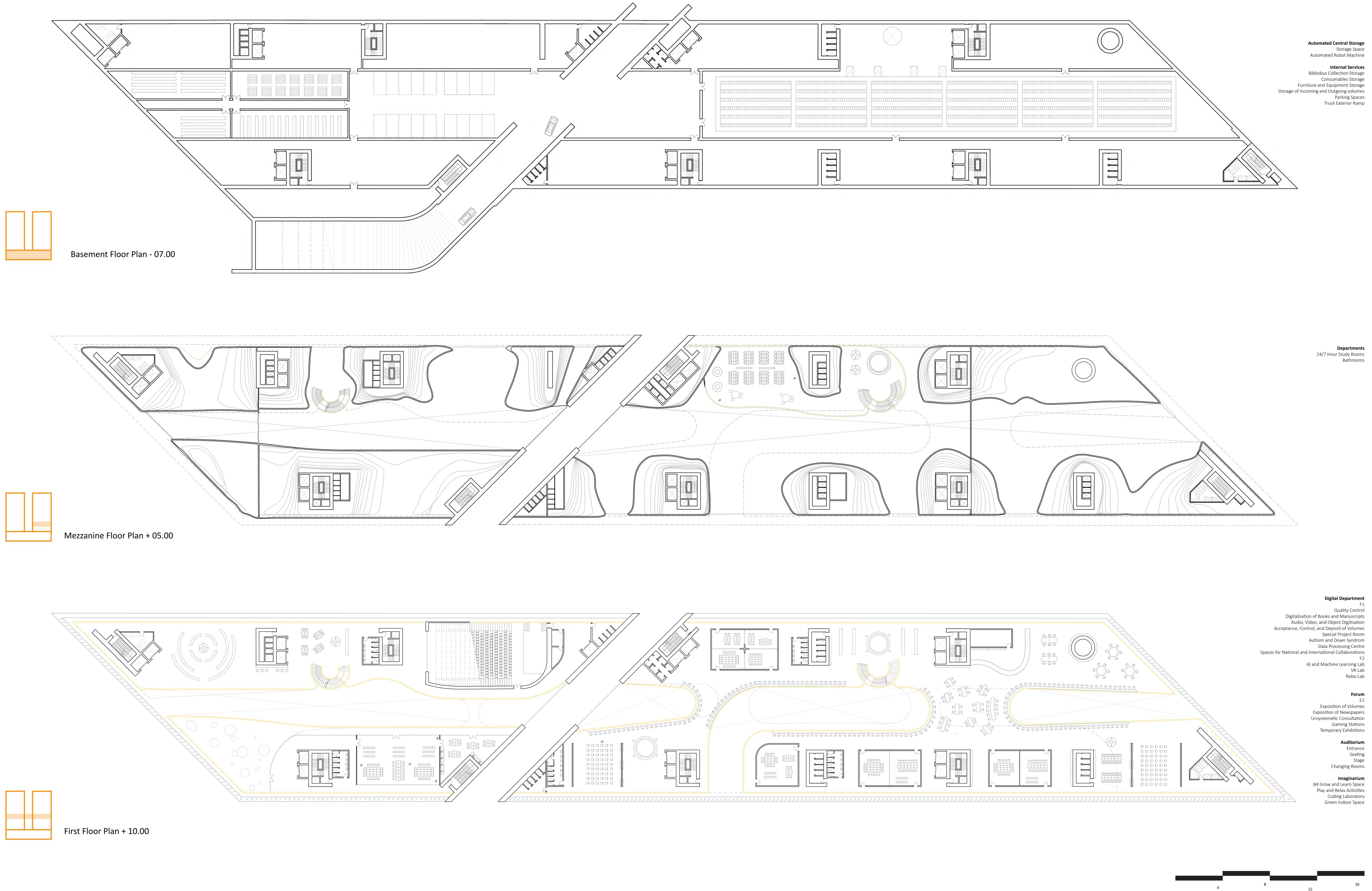


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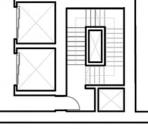
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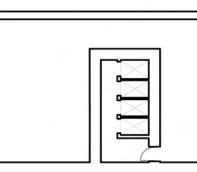
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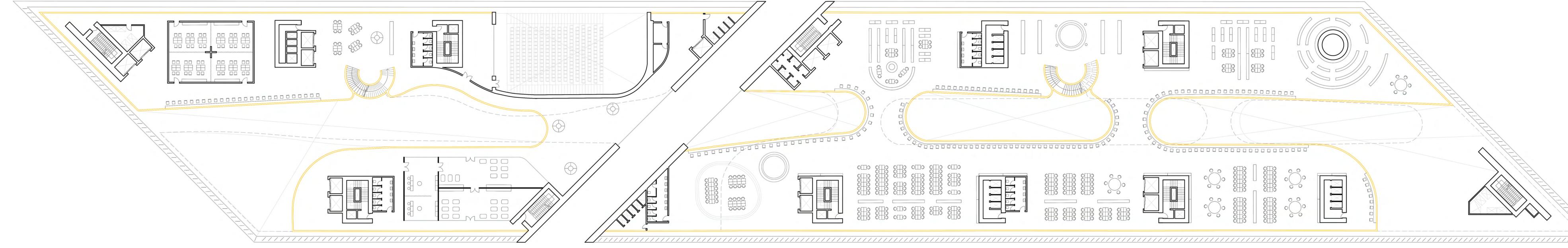


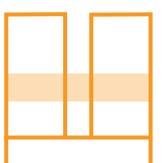




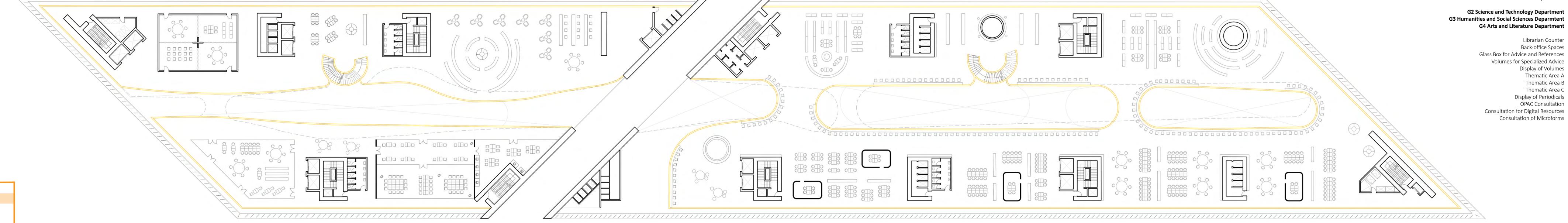
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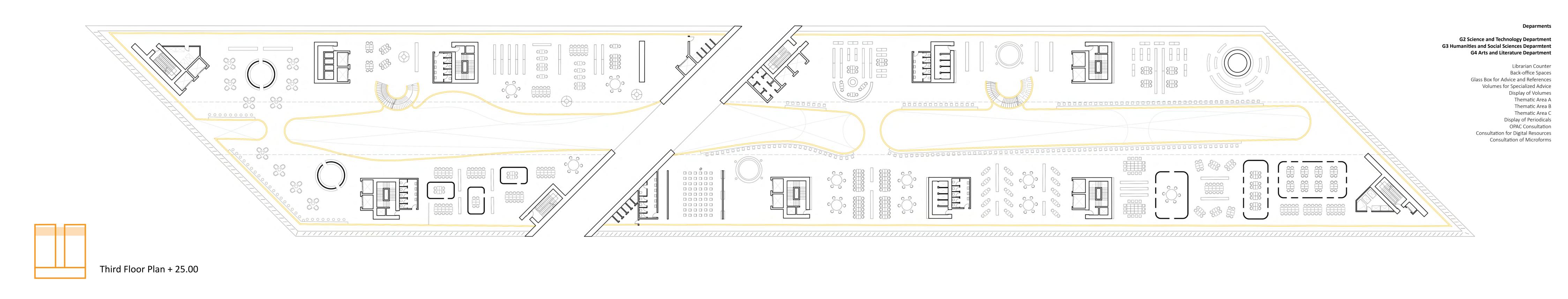




Second Floor Plan + 15.00









G2 Science and Technology Department umanities and Social Sciences Deparmtent G4 Arts and Literature Department Librarian Counter Back-office Spaces Glass Box for Advice and References Volumes for Specialized Advice Display of Volumes Thematic Area A Thematic Area B Thematic Area C Display of Periodicals OPAC Consultation Consultation for Digital Resources Consultation of Microforms

Deparmer

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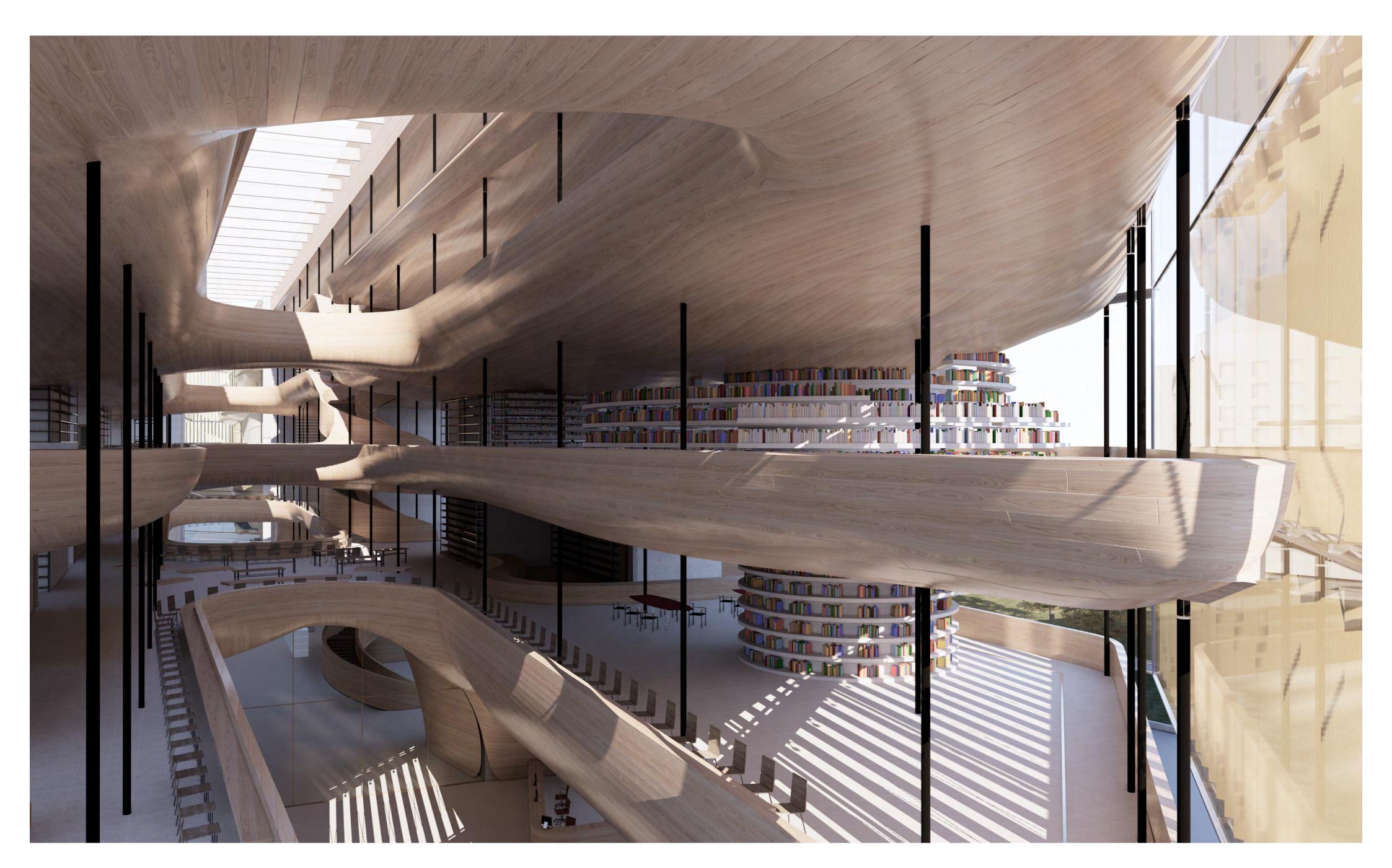
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Reading Spaces



Shelfing Spaces

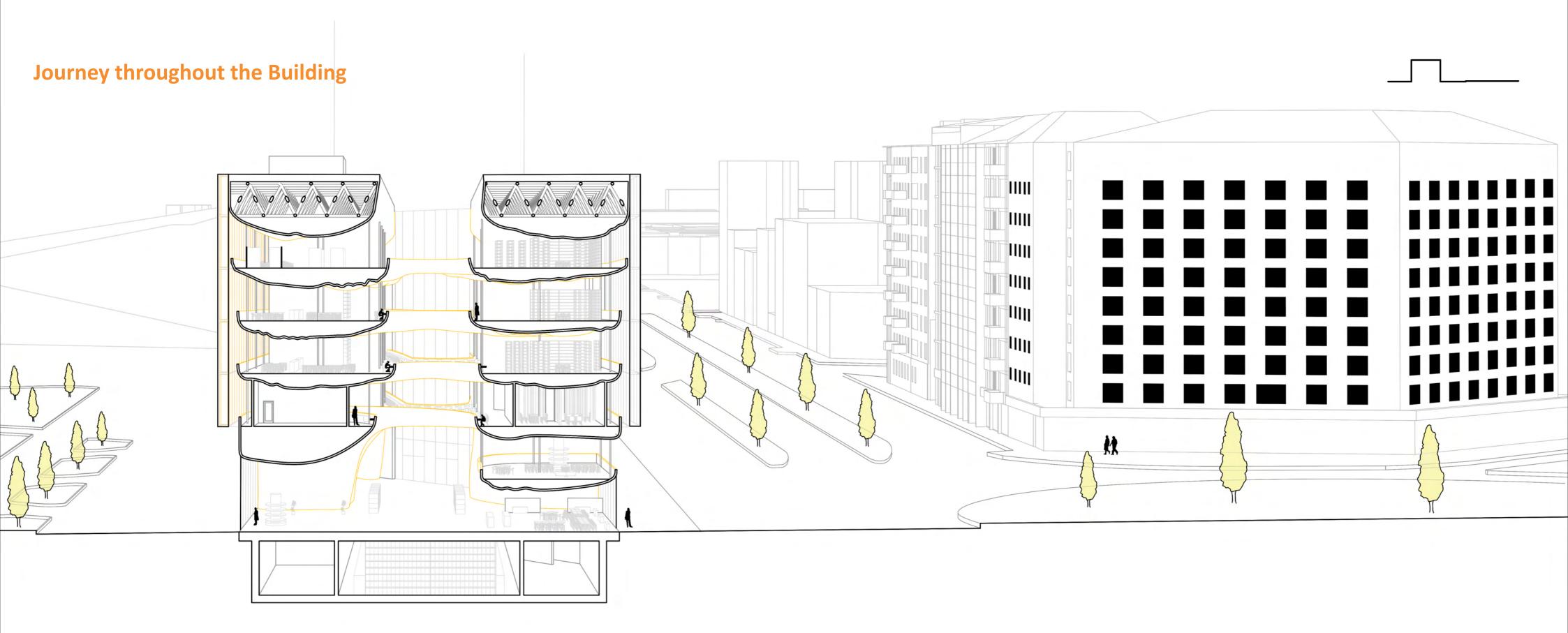
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Interior Views

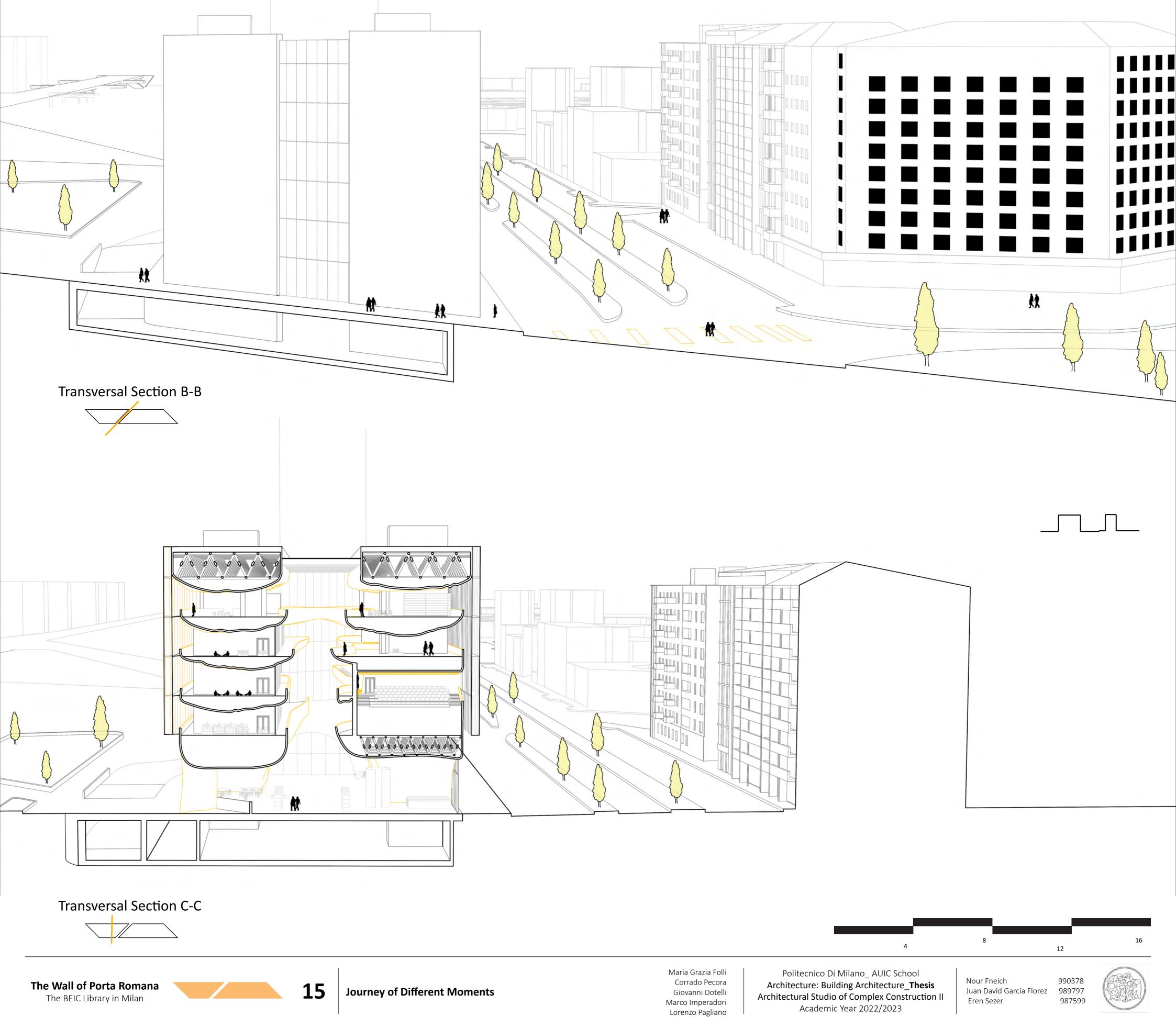
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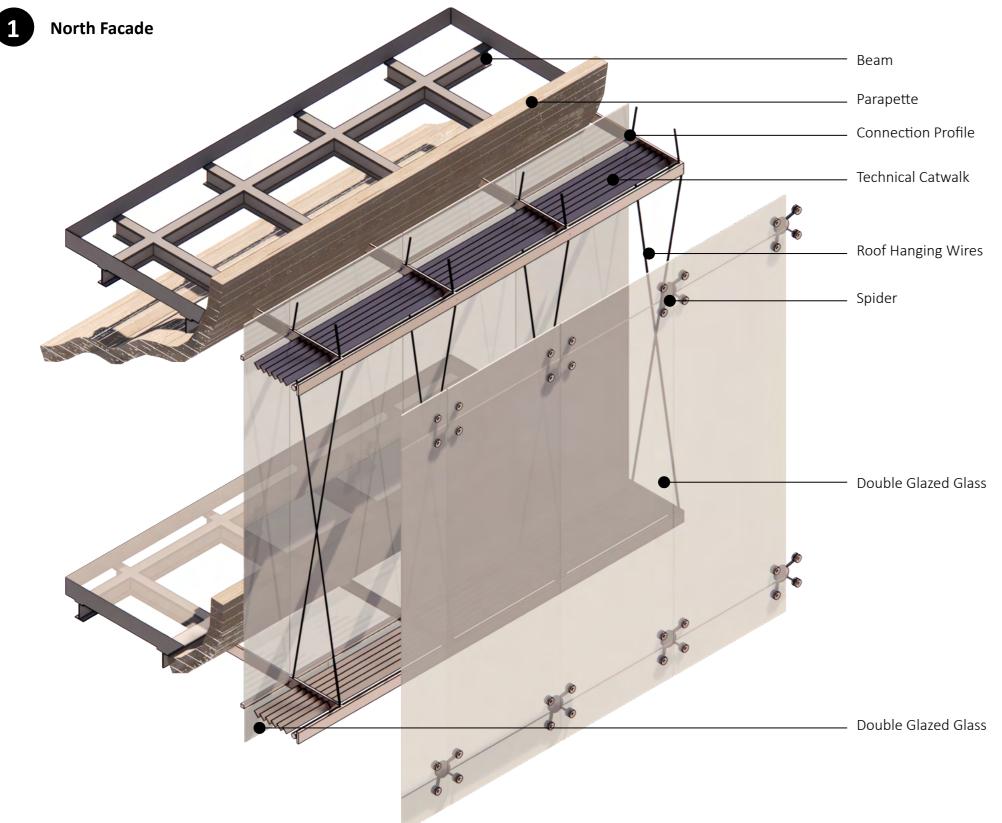
Transversal Section A-A



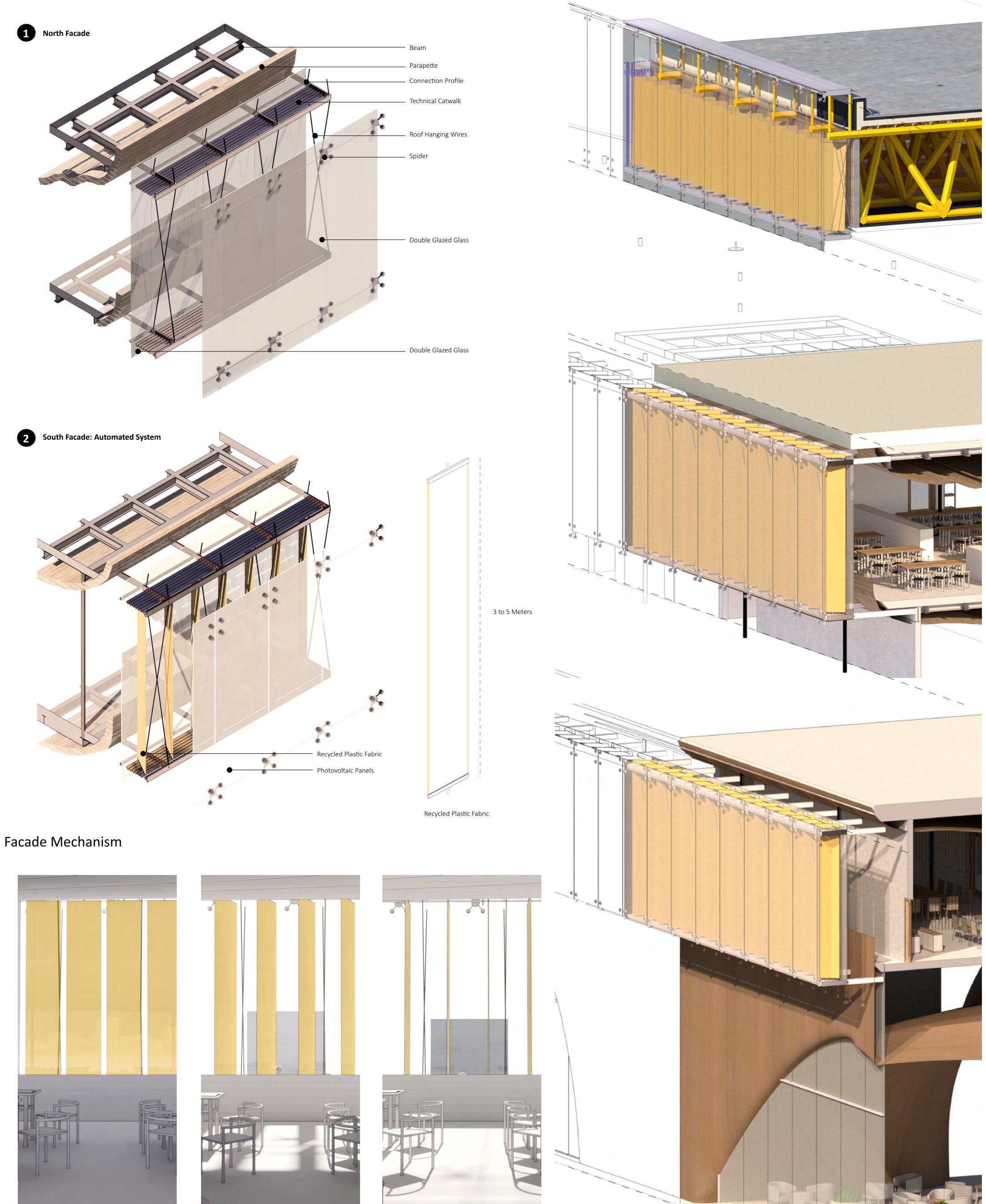


Facade Systems

Double Skin Facades



Facade to Structure Relation



Daylight Goal 6%

The Wall of Porta Romana

The BEIC Library in Milan



Facade System

Daylight Goal 13%

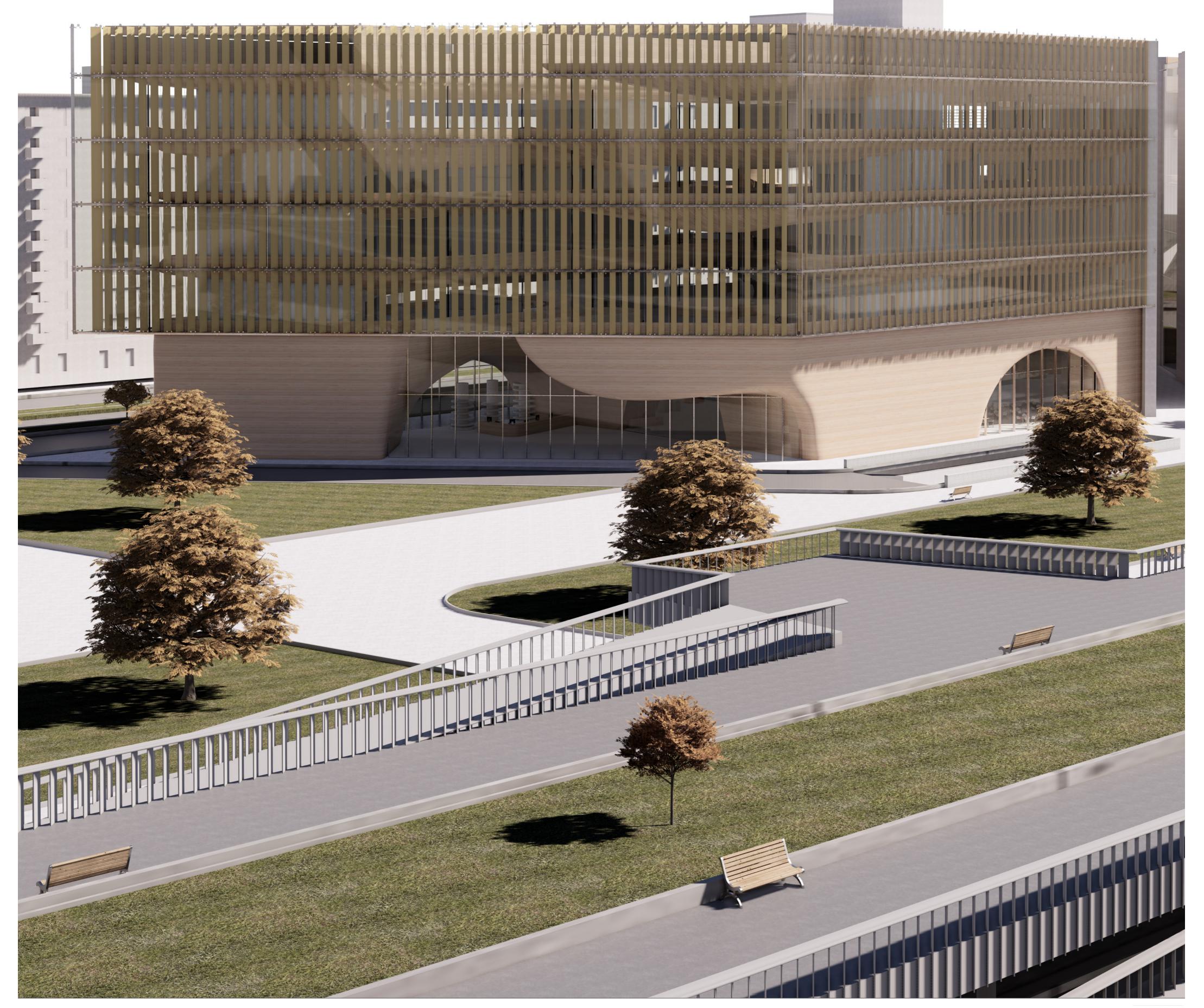
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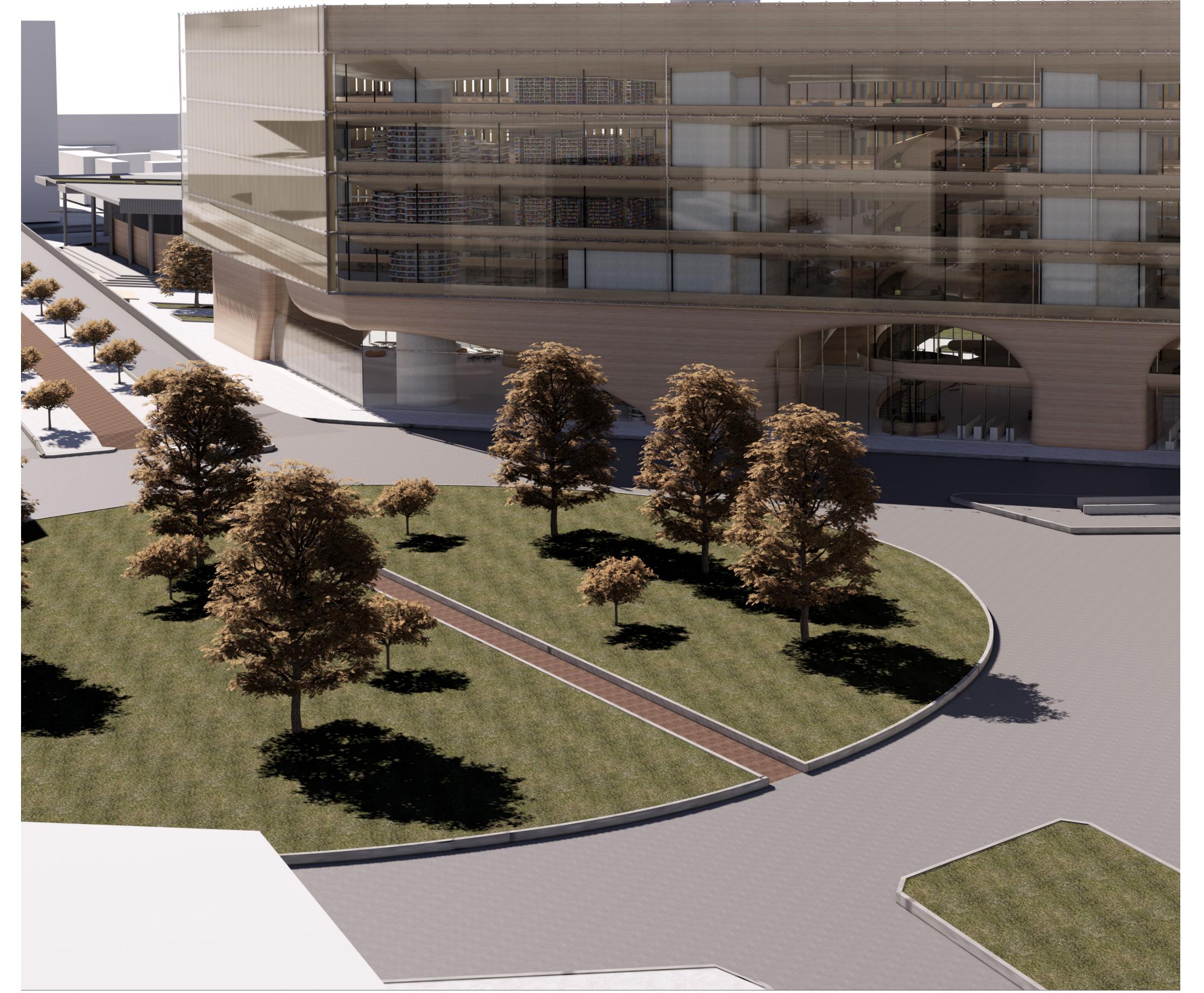


Building Facade

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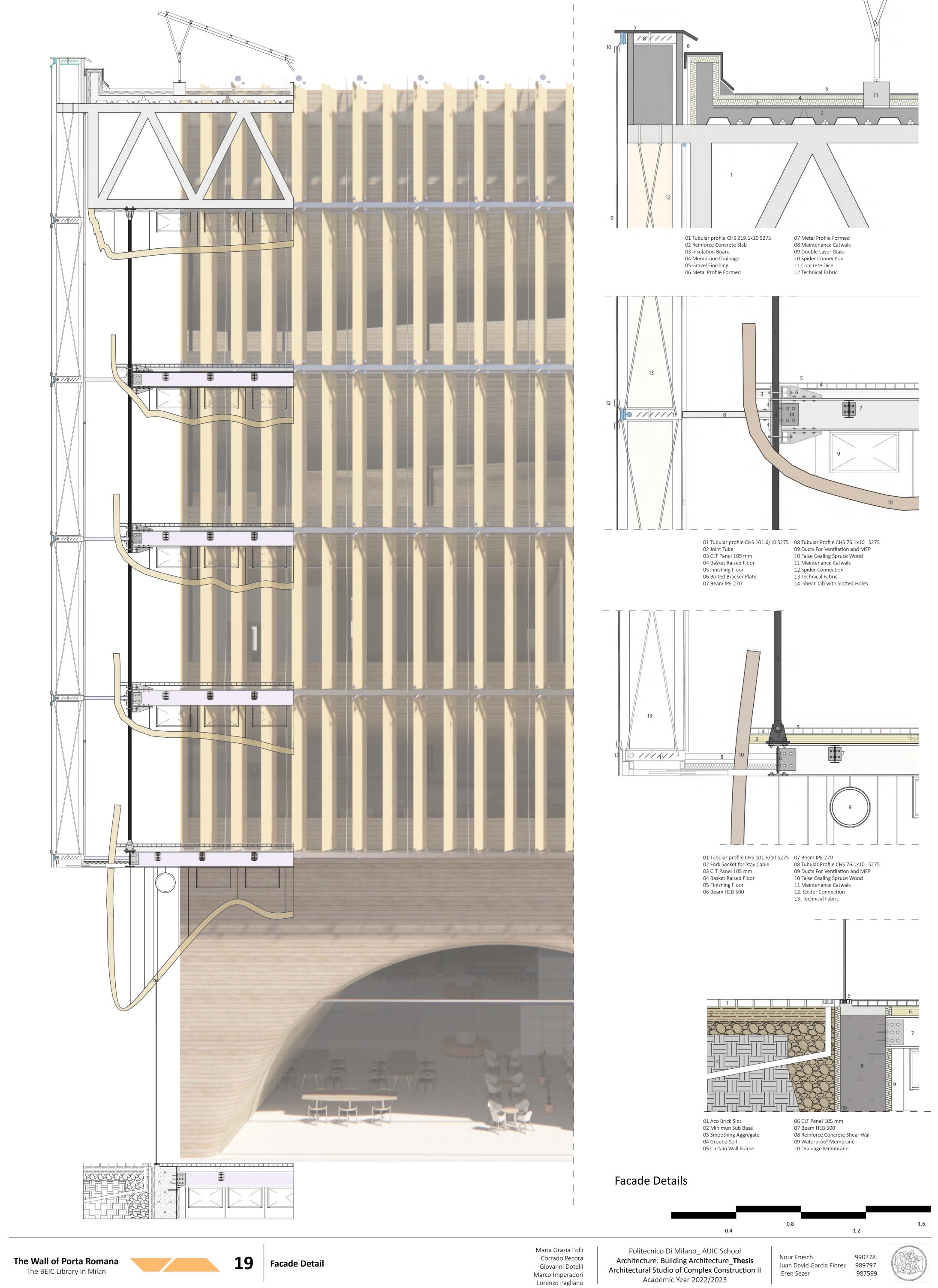
Building Facade

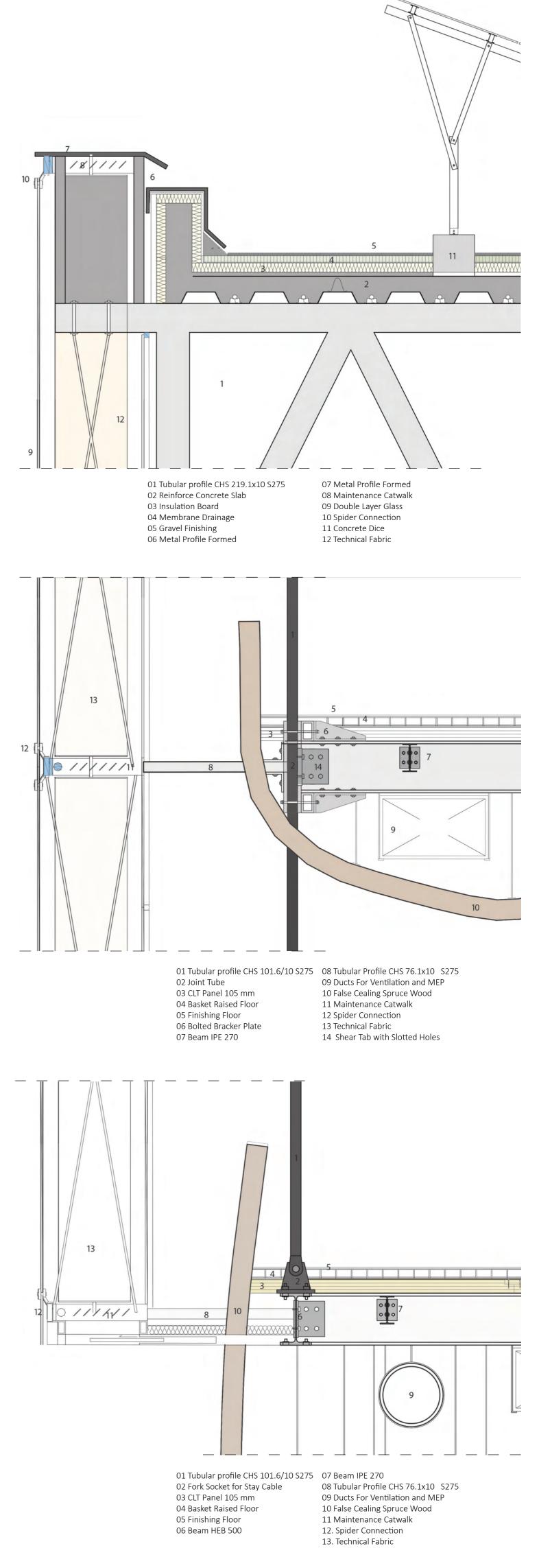
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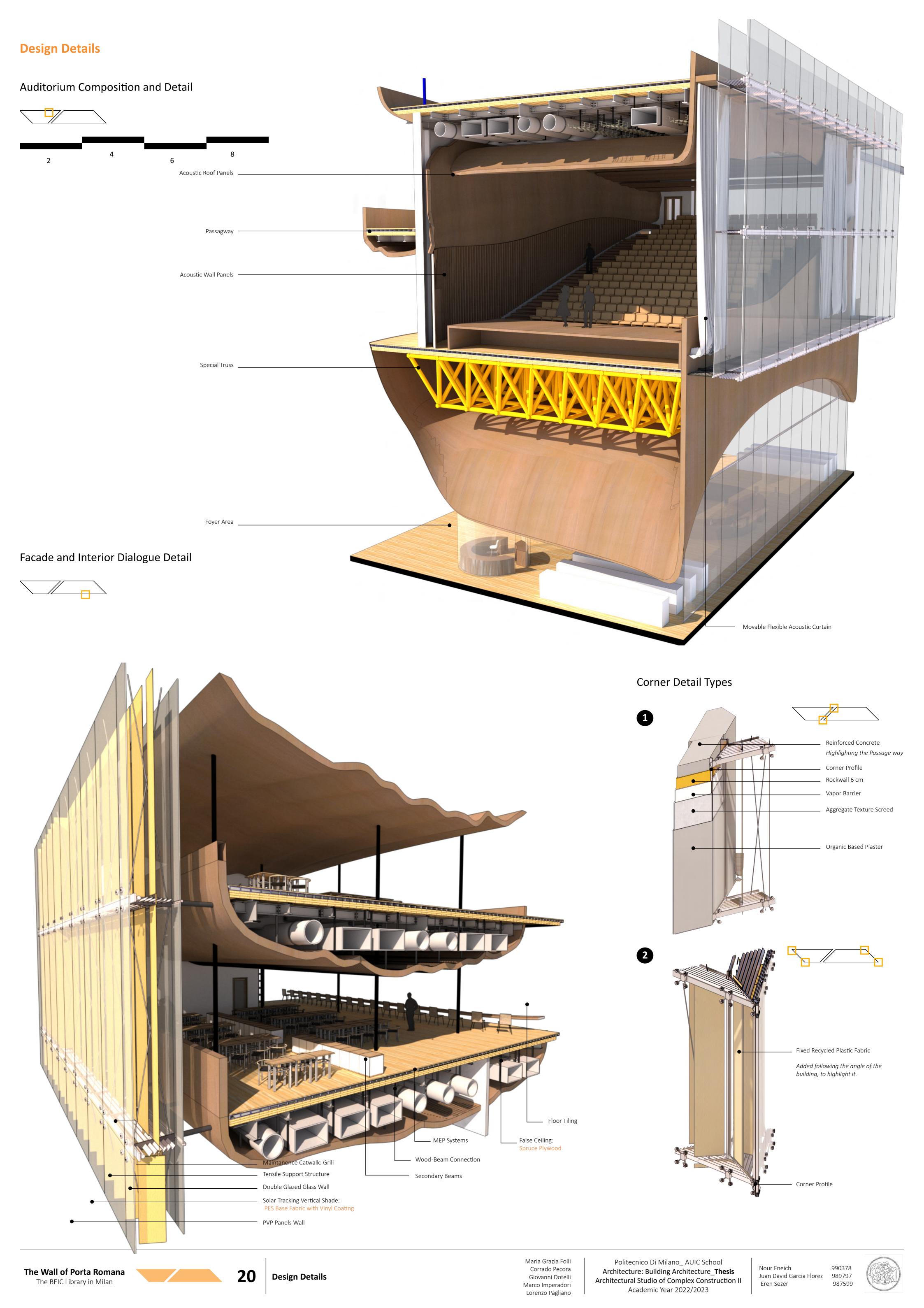
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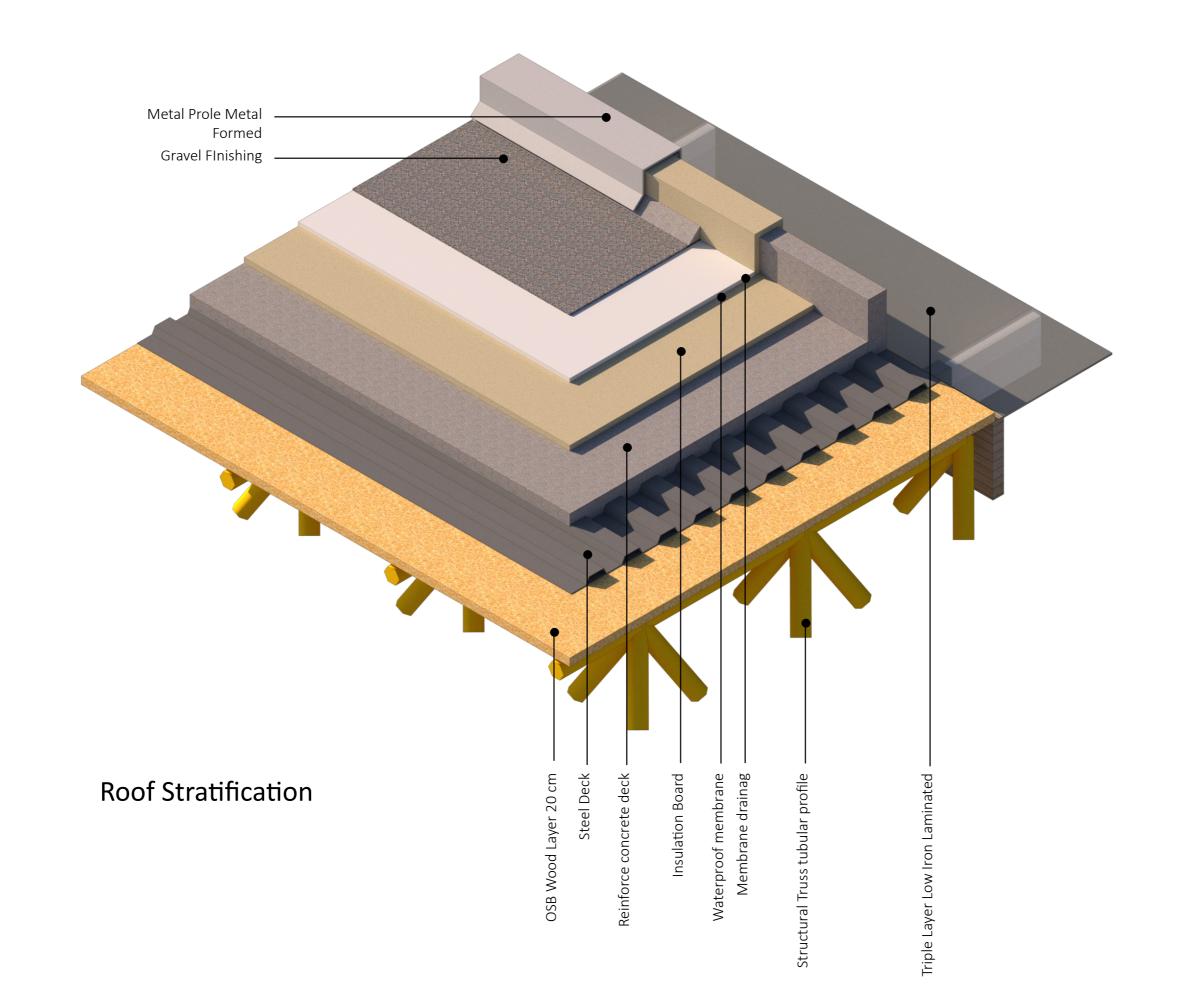
Building Facade: Details to Materials

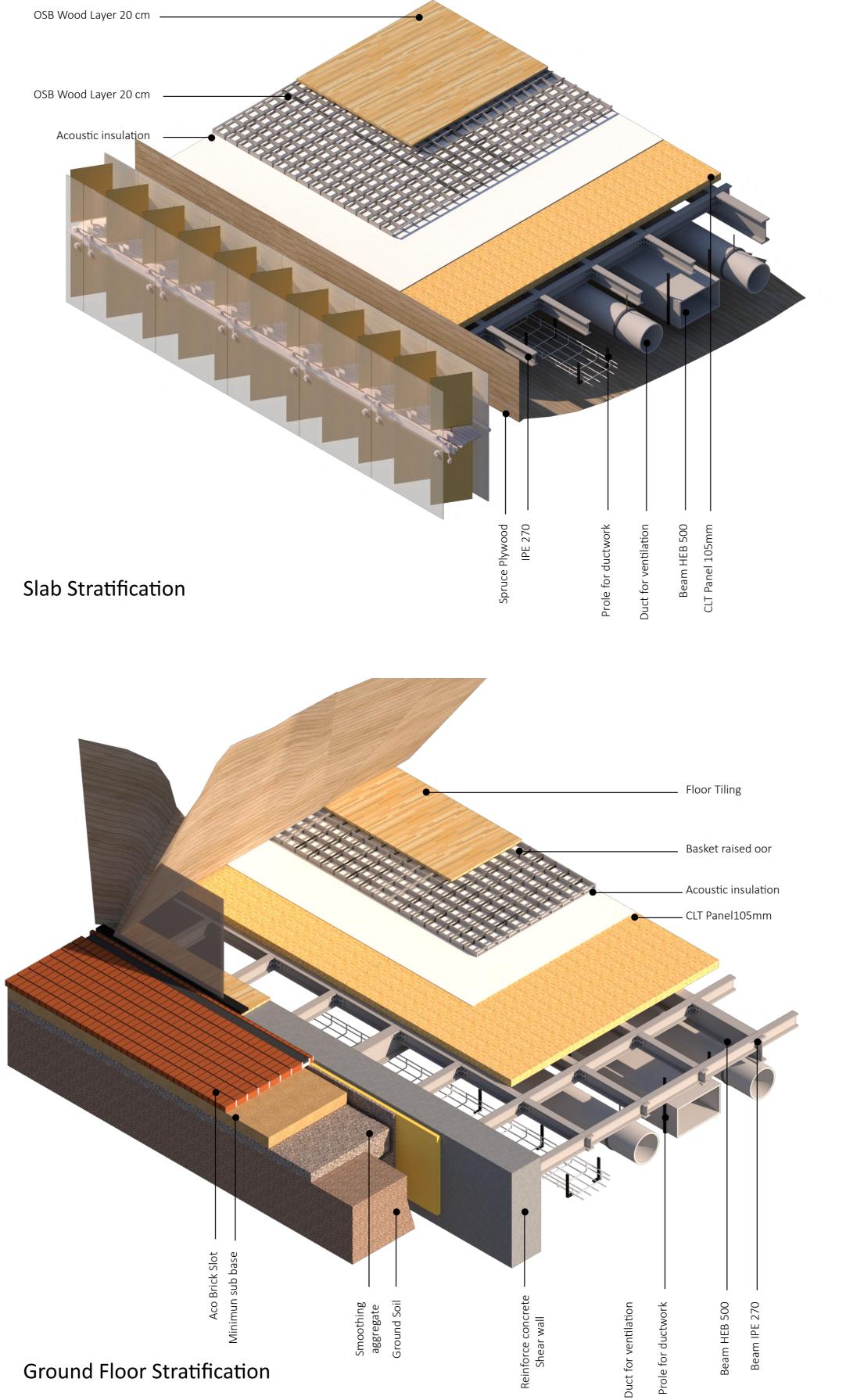






Materials and Stratification





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Materials and Stratification

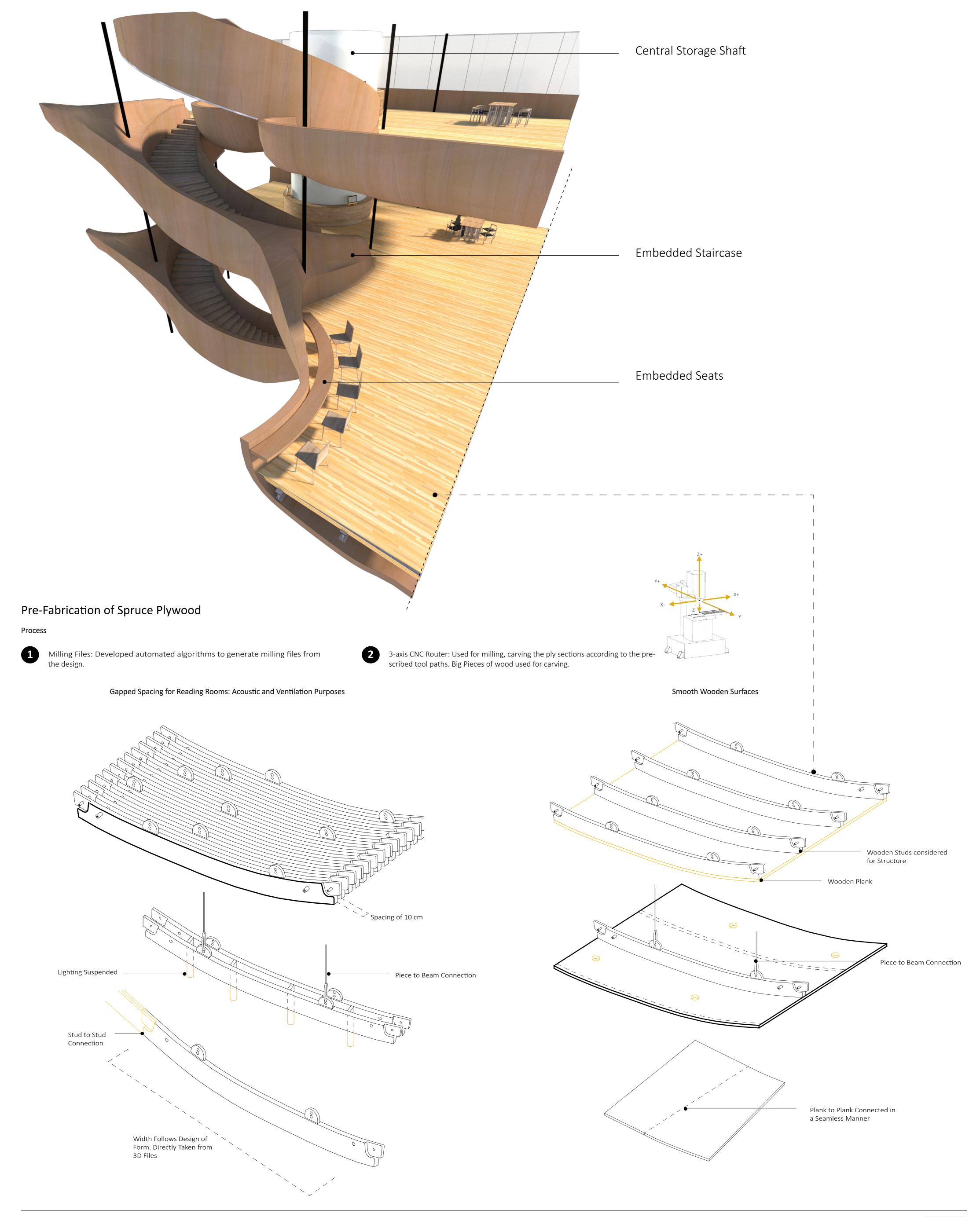
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Fabrication

Interior Carving: Material and Procedure

Sustainably-forested spruce plywood: Non-toxic water-based glue.



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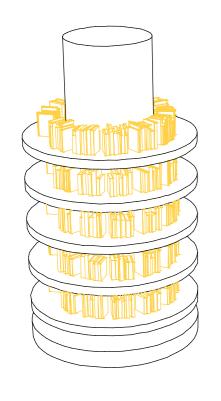
Fabrication: Interior to Furniture

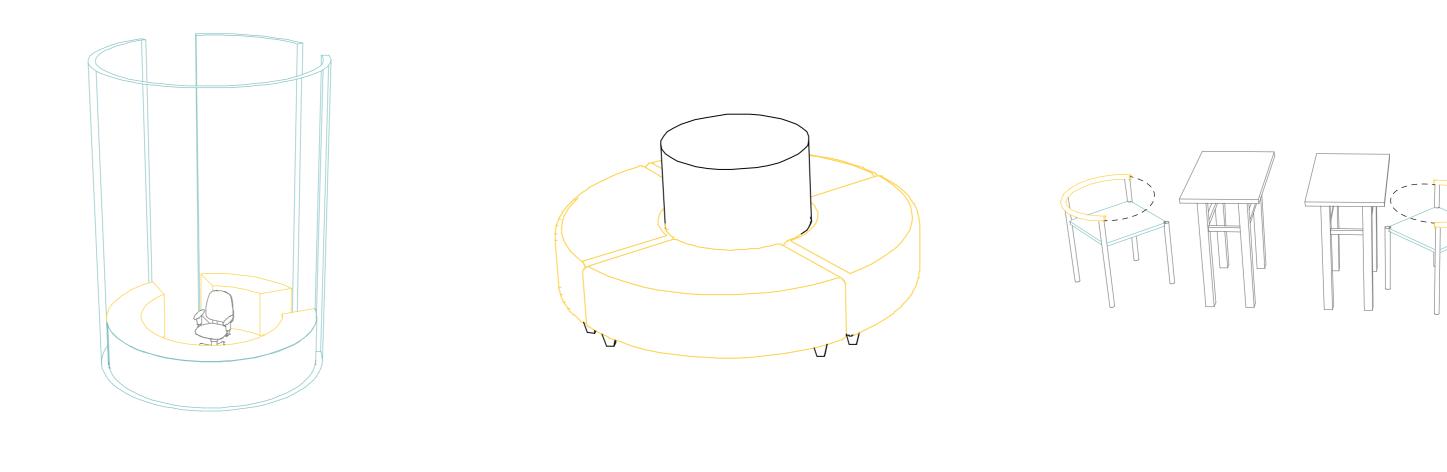
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Furniture pieces of BEIC Library: Designed according to the The Wall of Porta Romana Building Language



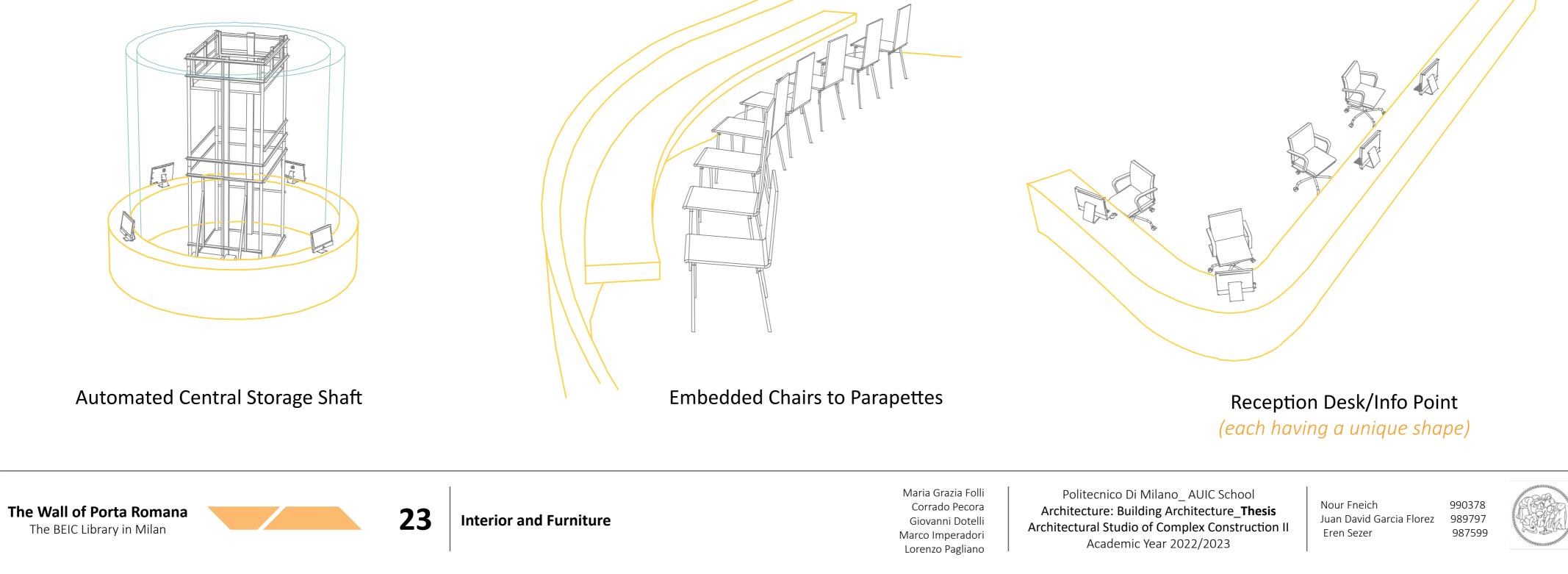


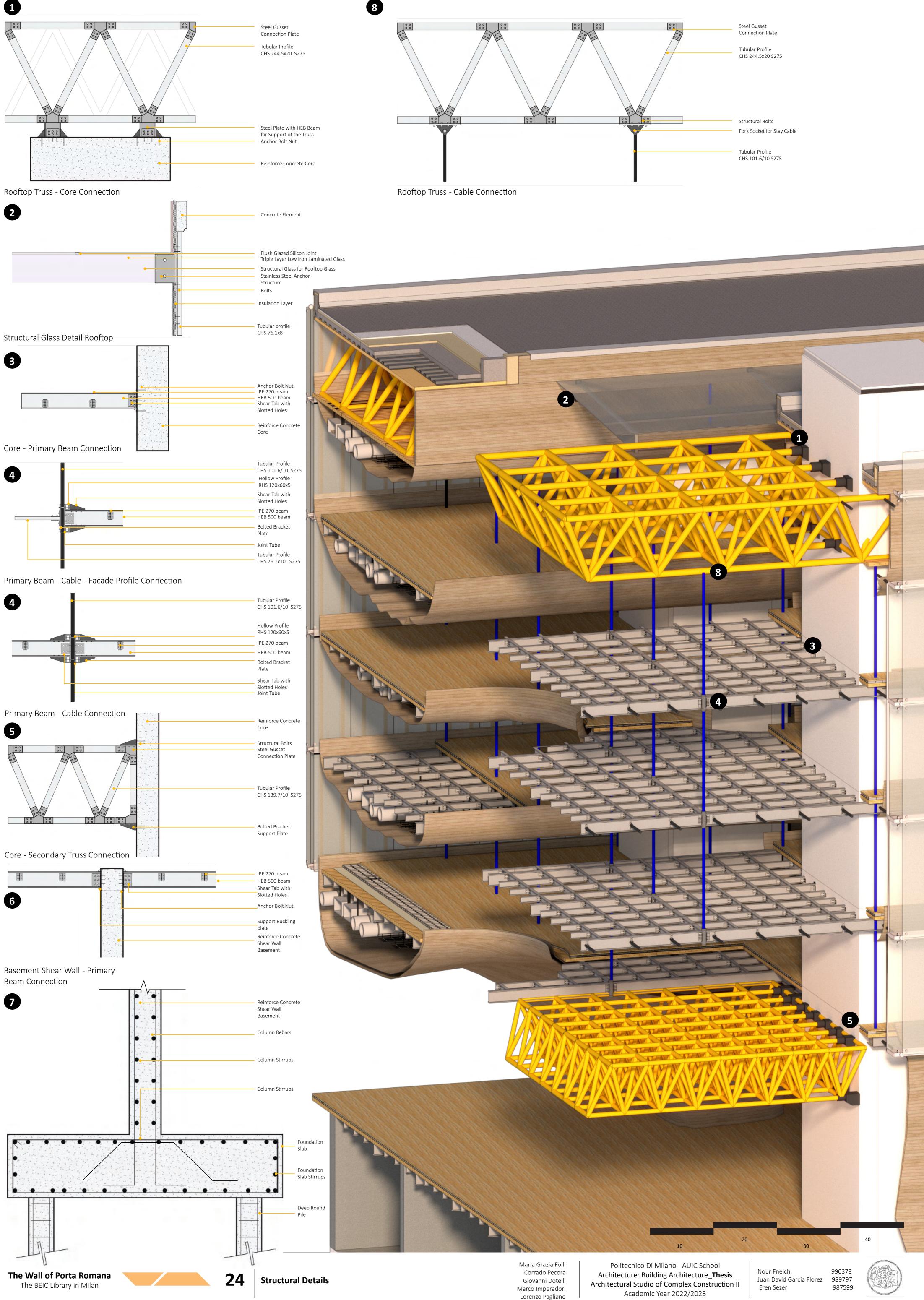
Column Bookshelves

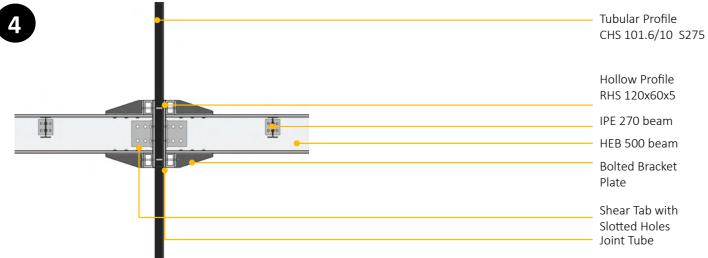
Translation Booths

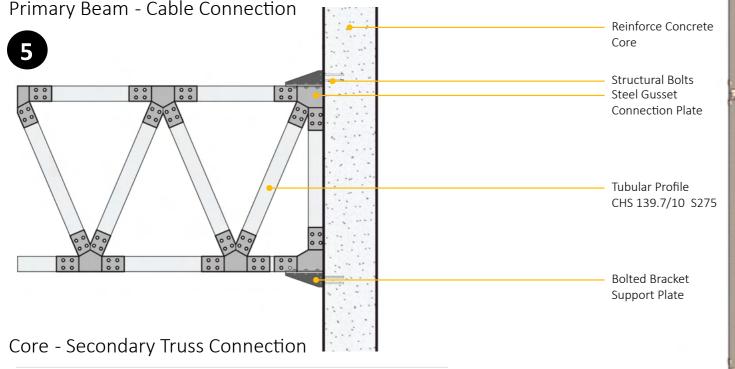
Seat Column

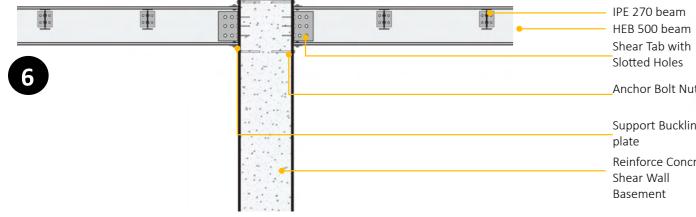
Modular Chair and Desk



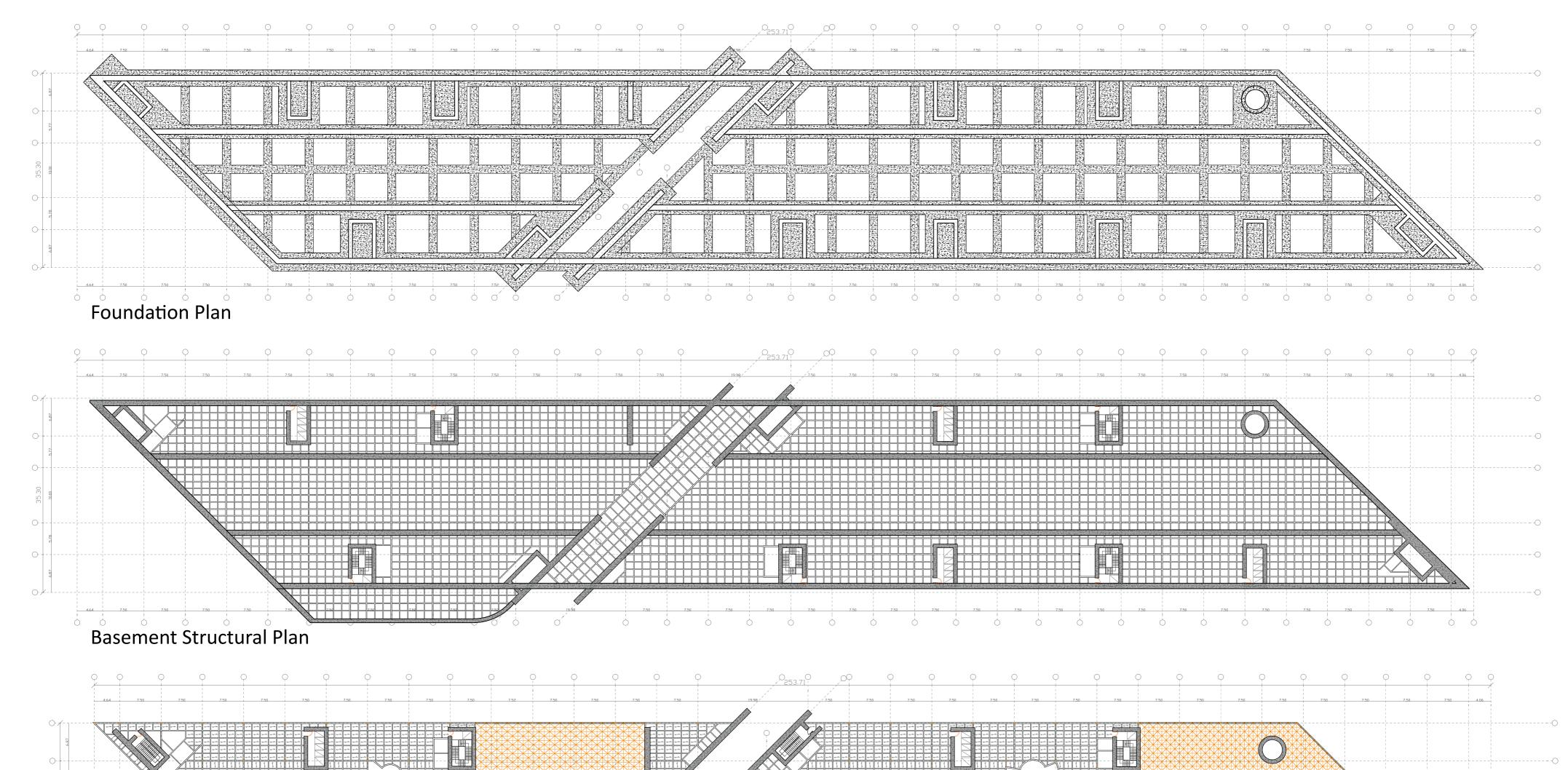






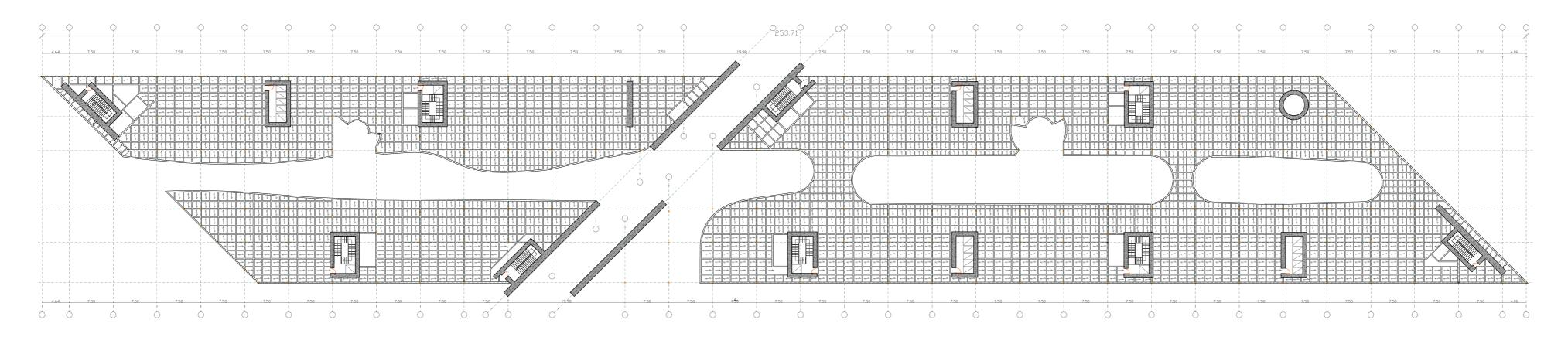




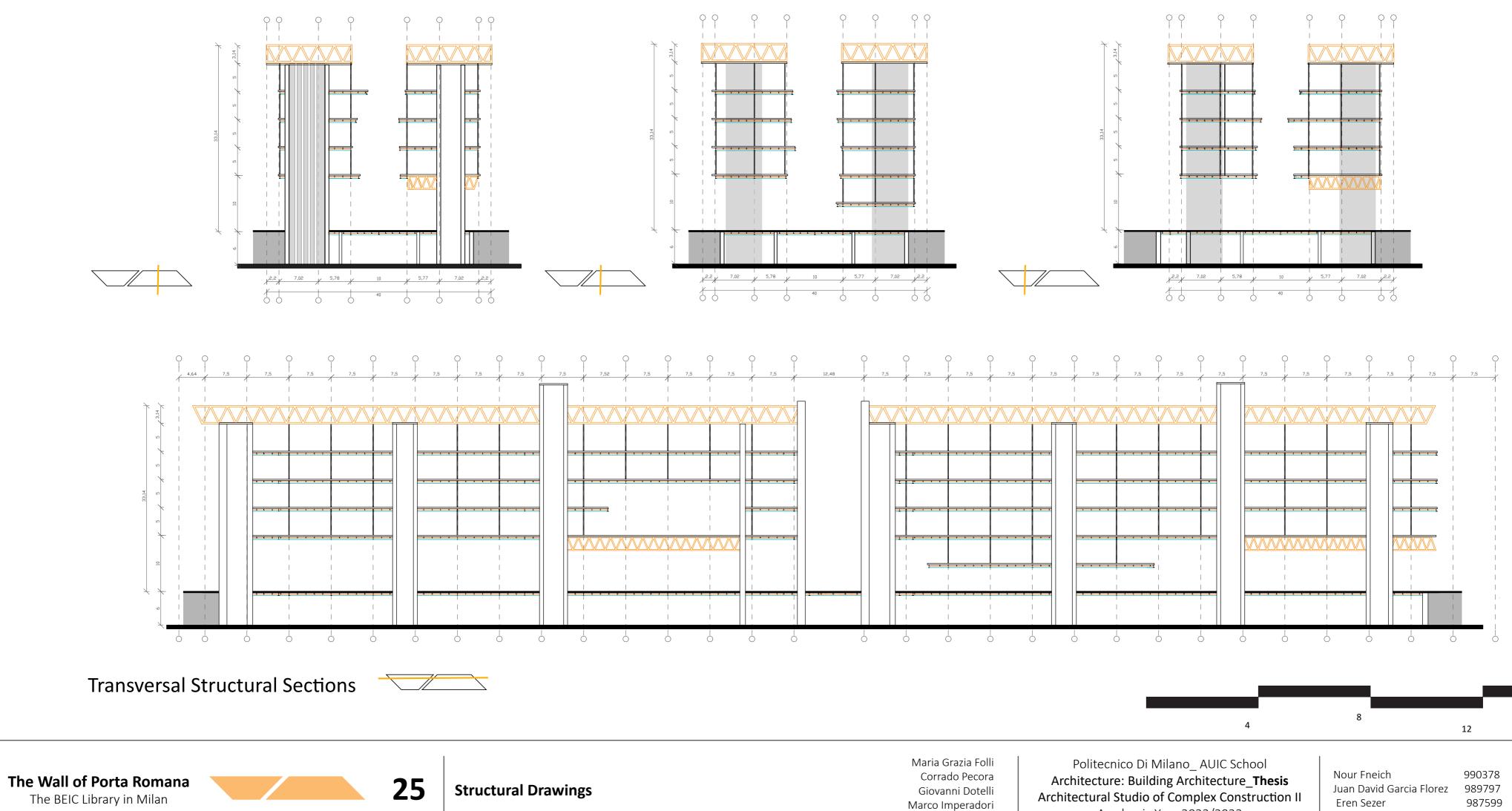


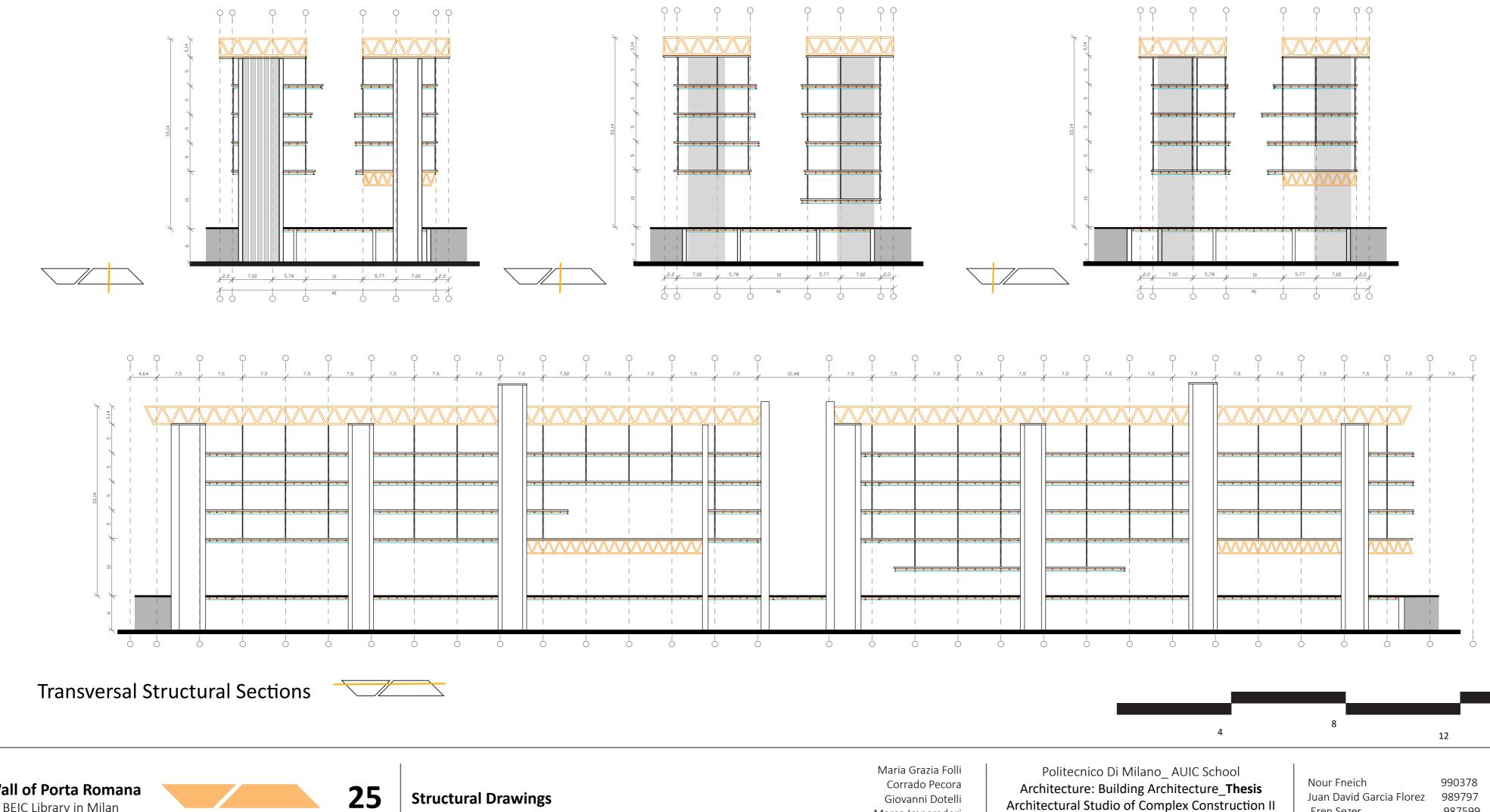


First Floor Structural Plan



Typical Floor Structural Plan





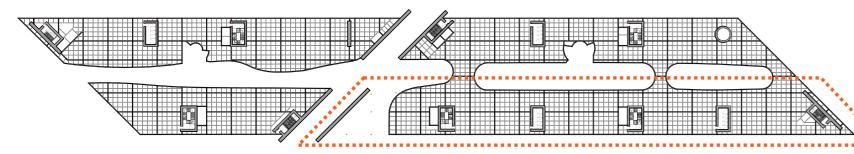
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Academic Year 2022/2023

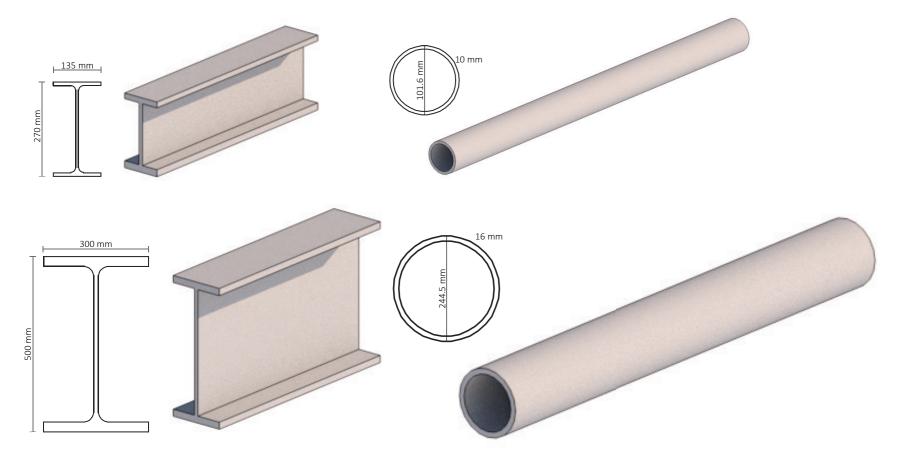
Lorenzo Pagliano



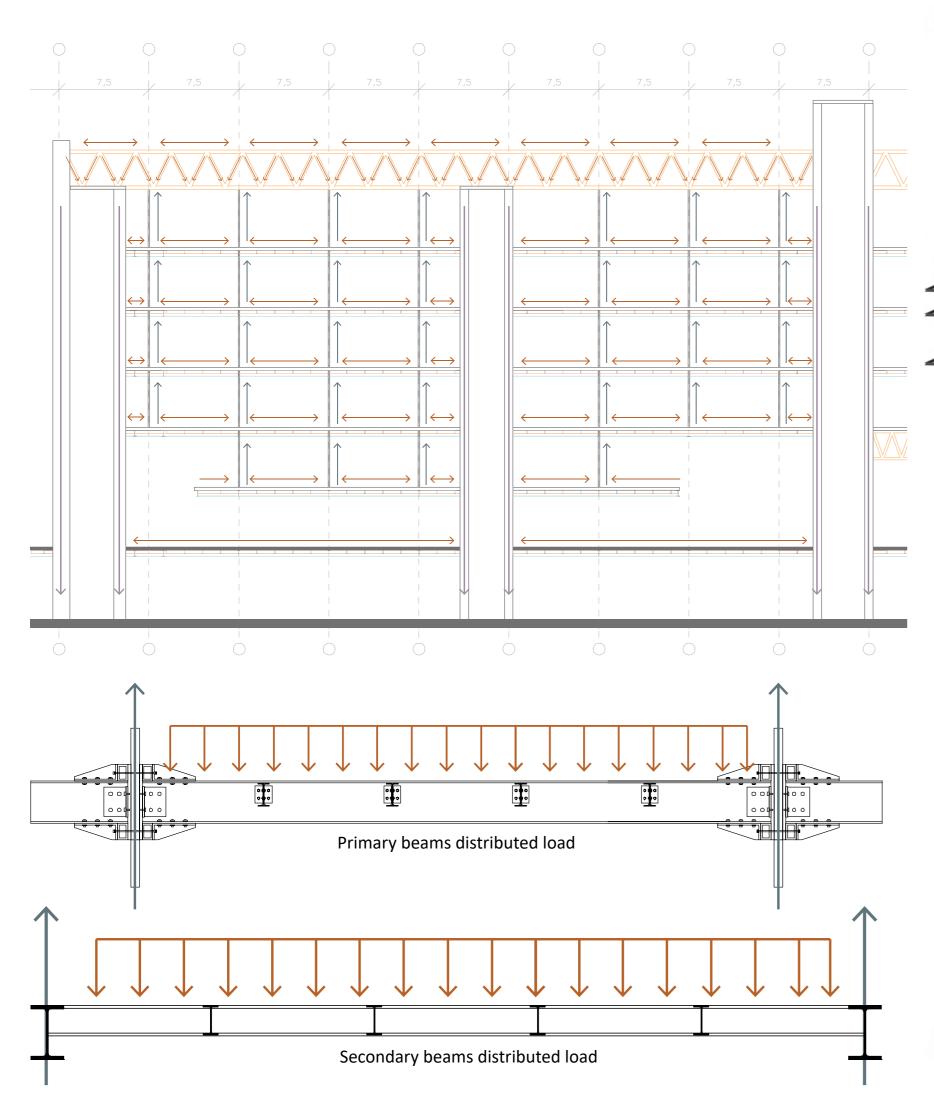
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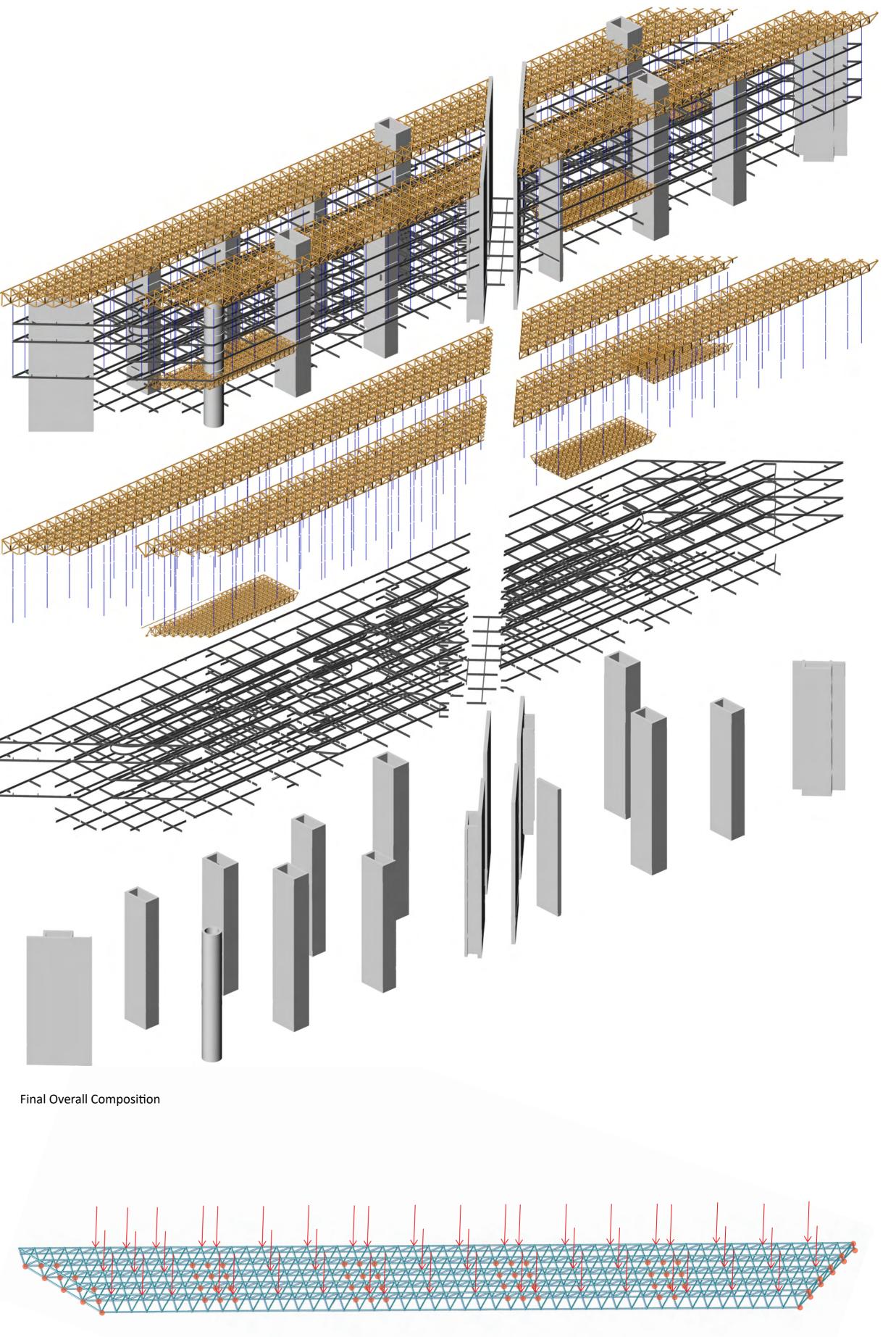


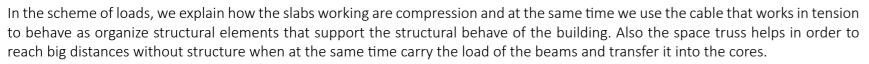
The selected area is an area that comprises different singularities within the structure, but at the same time it is the one that most closely represents the behavior of the structure and the loads encountered. In this area we can define how the structure behaves as a beam and the different cores as supports to understand the behavior of this system.

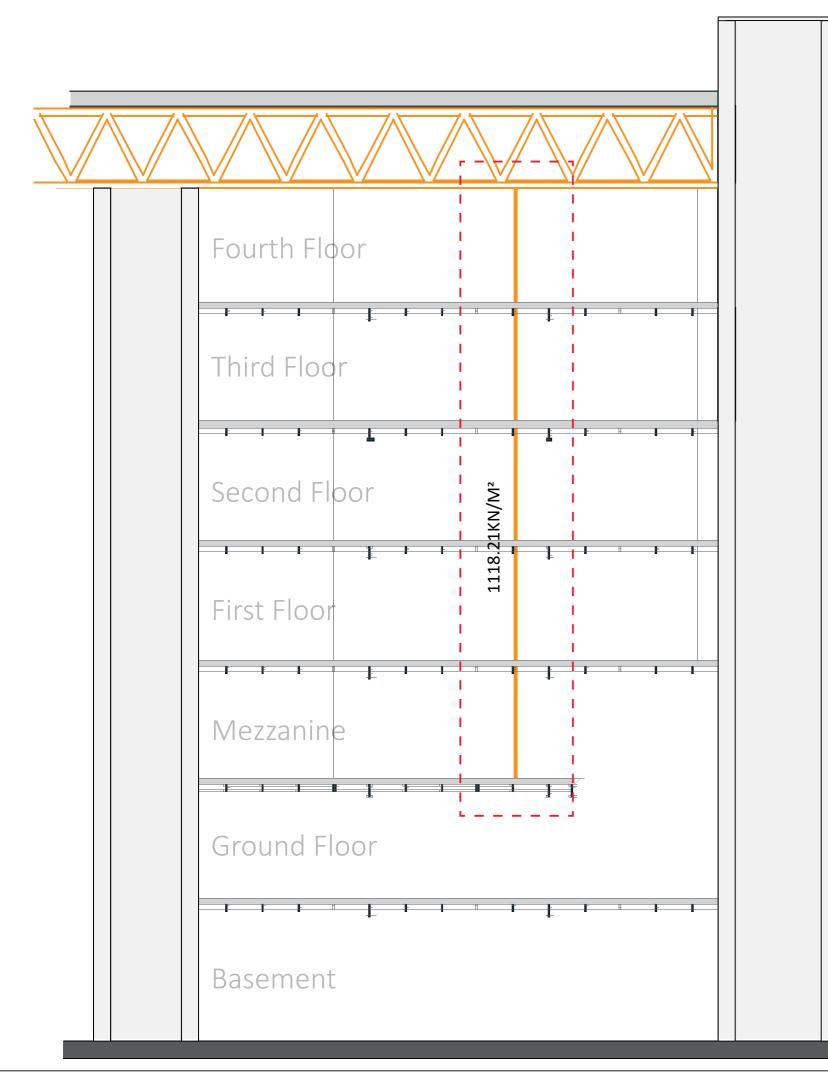


The structural elements used in the project are composed of primary and secondary beams with HEB 500 and IPE 270 respectively. The slabs in order to make them lighter, we use CLT for it characteristics and behavior. Also, in the decisions of the materials, we use steel tubular profiles CHS 244.5x20 for the space truss.









In the first part of the analysis made by MIDAS, we evaluated all the structural elements of the space truss located in the rooftop of the building. The model was imported from Archicad to Autocad and then to MIDAS.

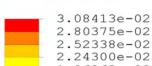
With this configuration we also include the distributed load of the rooftop that included the dead load of 6 kN/m²(DL) where is the structure and the slab . Also we included the snow load (SL) that have a load of 1.5 kN/m^2 .

Puntual Load- 1118.21 kN/m²

Restrains- Connection Truss / Cores

ULS Ultimate Limit State 1.3 x (DL) + 1.5 x (LL) SLS Serviceability Limit State 1.0 x (DL) + 1.0 x (LL)

Displacement - SLS 1.0 x (DL) + 1.0 x (LL) CHS 244.5 x 20



RESULTANT

1.96263e-02 1.68225e-02 1.40188e-02 1.12150e-02 8.41126e-03 5.60751e-03 2.80375e-03 0.00000e+00

At the beginning of the model, the restrains that connect the cores and the truss were fixed about displacement and free about rotations, also for the model we did not include any hinge to make connections between the bars. In the displacement analysis we can see how in the zone where the greatest deformation is found, is the right zone, where the greatest distance between supports is found, in addition to having the same amount of point loads. this displacement is 0.03 m or 3 cm.

MOMENT-y MOMENT in Y - ULS 3.97582e+01 3.02759e+01 1.3 x (DL) + 1.5 x (LL) 2.07936e+01 1.13112e+01 CHS 244.5 x 20

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Structural Verification

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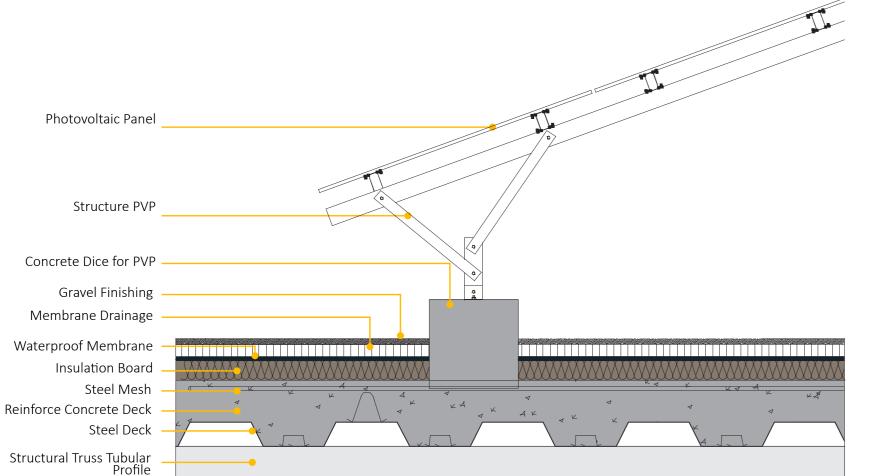
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-2.66182e+01 -3.61005e+01 -4.55828e+01 -5.50652e+01 -6.45475e+01

In the calculation of the weight of the slabs, we can appreciate the use of the CLT and the only slab that have different characteristics is the rooftop, in which we use a reinforce concrete slab, this due the behave of the CLT slabs in outside conditions like rain, ice and sun. Also, the idea is to create light structure with the same concept for the slabs to control the dimension of the elements, in order to reduce costs have a better composition in the project.



Typical Rooftop

		Rooftop Slab		
Dead	Loads			
G1	Space truss	Circular Hollow section	0,294	[kN/m
		139,7 mm Diameter 10 mm thick	ness	
G1			0,294	[kN/m]
G2	Slab Distributior	1		
		PVP with structure (aprox)	0,50	[kN/m2]
		Gravel Finishing 2 cm deep	0,34	[kN/m2
		Waterproof membrane	0,00	[kN/m2
		Insulation Board	0,10	[kN/m2
		Reinforce concrete deck	4,80	[kN/m2
		Steel Deck	0,16	[kN/m2
		Insulation board	0,10	[kN/m2
		Total	6,00	
G2 Pri	ncipal Beam		44,978	[kN/m]
G2 Se	condary Beam		8,276	[kN/m]
Q	Live Loads Libra	γ		
		Linear Load Library	4,0	[kN/m
		Snow Load Library	1,3	[kN/m
		Total	5,30	
G2			39,8	[kN/m

The roof has a construction system of a reinforced concrete slab, while steel deck sheets are used. This in order to avoid any type of water ingress. In addition to these elements, waterproof membranes are used. This structure is also designed to support the multiple photovoltaic panels that are installed on the roof, in order to supply

AXIAL 1.84157e+03 L.48409e+03 **AXIAL FORCE - ULS** 1.12661e+03 7.69127e+02 1.3 x (DL) + 1.5 x (LL) 4.11646e+02 0.00000e+00 CHS 244.5 x 20 -3.03316e+02 -6.60797e+02 -1.01828e+03 -1.37576e+03 -1.73324e+03 -2.09072e+03 MOMENT-z MOMENT in Z - ULS 1.3 x (DL) + 1.5 x (LL) CHS 244.5 x 20

2.75393e+01 2.29636e+01 1.83879e+01 1.38122e+01 9.23646e+00 4.66076e+00 0.00000e+00 -4.49065e+00 -9.06636e+00 -1.36421e+01 -1.82178e+01 -2.27935e+01

SHEAR-z

SHEAR in Z - ULS 1.3 x (DL) + 1.5 x (LL) CHS 244.5 x 20

Displacement - SLS

1.0 x (DL) + 1.0 x (LL)

CHS 244.5 x 16

2.75836e+01 2.17430e+01 1.59023e+01

1.00617e+01 4.22109e+00 0.00000e+00 -7.46016e+00 -1.33008e+01 -1.91414c+01 -2.49820e+01 -3.08227e+01 -3.66633e+01

DISPLACEMENT

RESULTANT

3.71805e-02

3.38005e-02

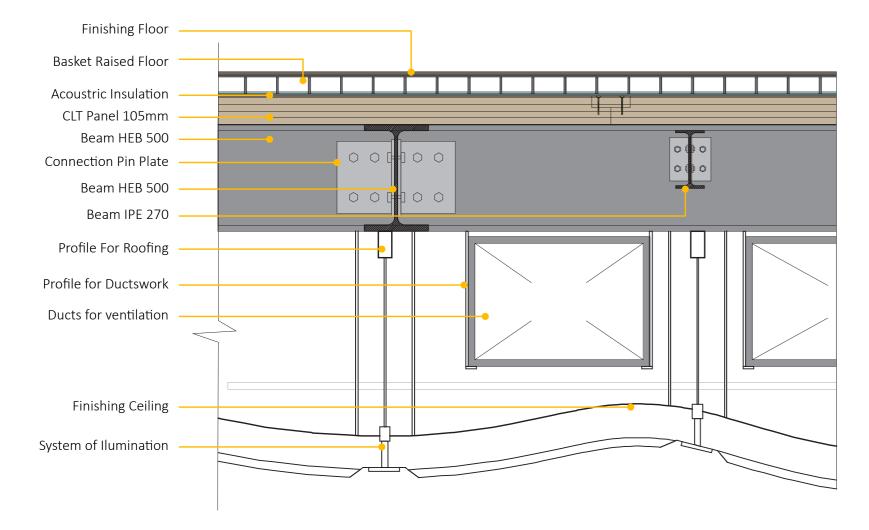
3.04204e-02

2.70404e-02

2.36603e-02 2.02803e-02 1.69002e-02

1.35202e-02

In making the different analyses, tests were also carried out with other sections, looking for the best configuration according to the behavior and the results of the analysis. One of these comparisons was with the execution of the analysis with CHS section 244.5x16 and section 216.1x20. With the first section we see how the displacement increases as the steel code checking result analysis shows that failures are present. With the second section, in spite of having the same thickness as the current one, due to the diameter, we find failures in the steel checking results, while generating a displacement similar to that found with the CHS244.5x16 section.



Typical Slab

		Typical Slab		
Dead I	Loads			
G1	Primary Beams	HEB 500	1,873	[kN/m
	Secondary Beams	IPE 270	0,361	[kN/m
G1			2,234	[kN/m
G2	Slab Distribution			
		PVC floor panel + System	0,26	[kN/m2
		Impact insulation layer	0,09	[kN/m2
		Acoustic insulation x2	0,06	[kN/m2
		CLT panel	0,44	[kN/m2
		AC system	0,39	[kN/m2
		Finishing - Wood Element	0,60	[kN/m2
		Total	1,84	
G2 Principal Beam			13,800	[kN/m
G2 Se	condary Beam		2,539	[kN/m
Q	Live Loads Library			
		Linear Load Library	4,0	[kN/m
				[kN/m
		Total	4,00	
G2			30,0	[kN/m

The slabs are composed of the use of main and secondary beams generating a framework. This also with the use of CLT panels with the function of reducing the load of the structure. These elements also with the idea of using materials and methods that are

sustainable.

SHEAR in Y - ULS 1.3 x (DL) + 1.5 x (LL) CHS 244.5 x 16

AXIAL FORCE - ULS 1.3 x (DL) + 1.5 x (LL) CHS 244.5 x 16

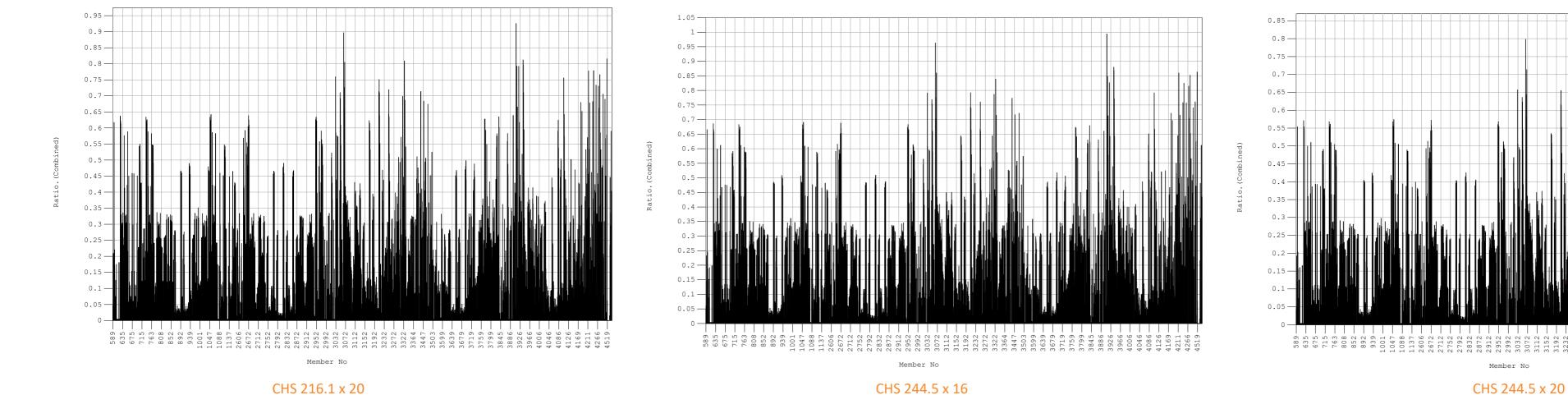
1.01401e-02 6.76010e-03 3.38005e-03 0.00000e+00
SHEAR-y
1.52957e+01 1.18851e+01 8.47454e+00 5.06395e+00 0.00000e+00 -1.75723e+00 -5.16782e+00 -8.57841e+00 -1.19890c+01

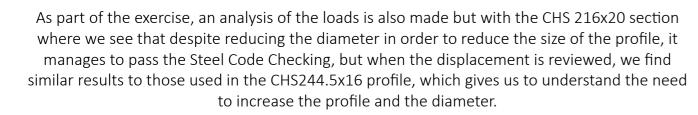
AXIAL

-1.53996e+01

1.88102e+01 2.22208e+01

2.34845e+03
1.89103e+03
1.43361e+03
9.76192e+02
5.18772e+02
0.00000e+00
-3.96066e+02
-8.53486e+02
-1.31091e+03
-1.76832e+03
-2.22574e+03
-2.68316e+03





In this analysis we see how this section (CHS2445x16) despite having a larger size, still presents failures in the steel code checking. This can also be seen in the deformation and displacament found in the analysis. By examining the options according to the need, we can see that even by increasing the section diameter, the result does not vary in comparison to the option of increasing the thickness.

The steel code checking shows how all the sections are below 1.0 and only a few are close to 0.85. The great majority is below 6 which gives us to understand how the structure, in spite of the section and the multiple loads it is subjected to, manages to resist. This also demonstrates how in the end, despite having a 3.0 cm deflection in the SLS, the structure performs adequately for the need.

Member No

The Wall of Porta Romana The BEIC Library in Milan

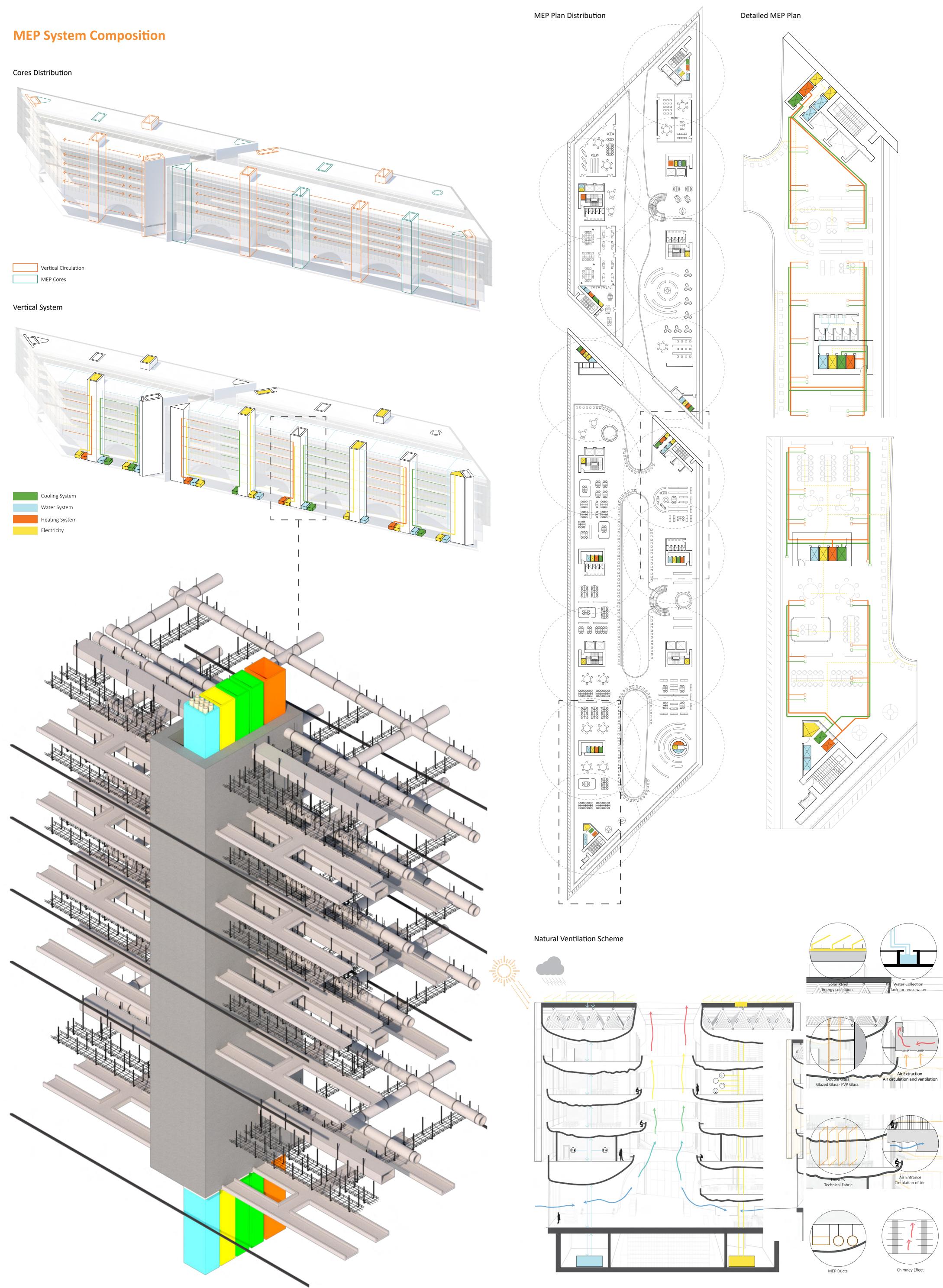


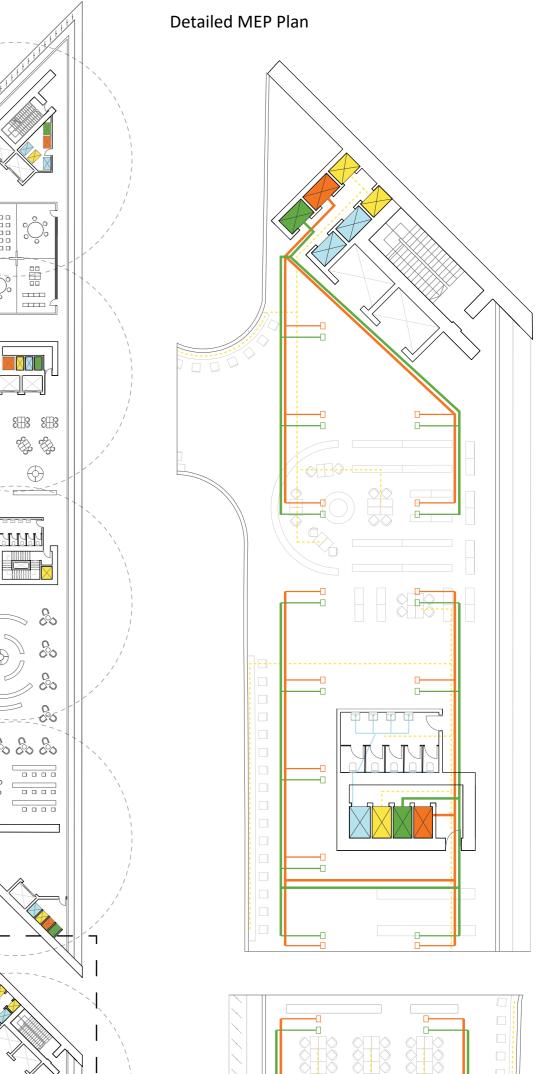
Structural Verifications

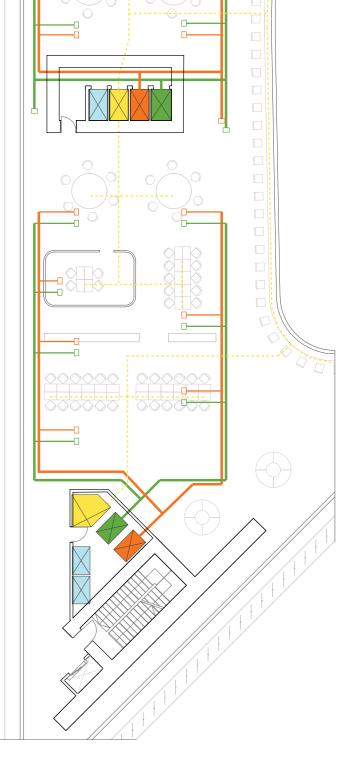
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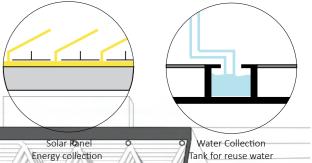
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